

WSU GOLF TEAM CLUBHOUSE UPGRADE EXTERIOR & ADDITION

DESIGN TEAM

OWNER

WASHINGTON STATE UNIVERSITY
PO BOX 641150
2425 EAST GRIMES WAY,
PULLMAN, WASHINGTON 99164-1150
TEL: (509) 335-9024

CONTACT:
JASON BAERLOCHER

ARCHITECTURAL

DESIGN WEST ARCHITECTS P.A.
254 EAST MAIN STREET
PULLMAN, WASHINGTON 99163
TEL: (509) 332-3113

CONTACT:
NED WARNICK

CIVIL

SYNTIER ENGINEERING
405 BRELSFORD DR STE C
PULLMAN, WASHINGTON 99163
TEL: (509) 339-6187

CONTACT:
TOM STERLING

STRUCTURAL

LSB CONSULTING ENGINEERS
523 E. THIRD AVENUE
SPOKANE, WASHINGTON 99202
TEL: (509) 323-9292
FAX: (509) 747-7115

CONTACT:
LARRY HARRIS

MECHANICAL

MSI ENGINEERS
LEGION BUILDING
108 N. WASHINGTON, SUITE 505
SPOKANE, WASHINGTON 99201
TEL: (509) 624-1050

CONTACT:
AARON DONNELLY

ELECTRICAL

MSI ENGINEERS
LEGION BUILDING
108 N. WASHINGTON, SUITE 505
SPOKANE, WASHINGTON 99201
TEL: (509) 624-1050

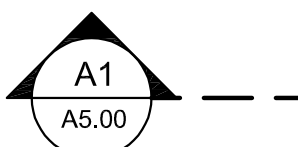
CONTACT:
BEN JENNINGS

WASHINGTON STATE UNIVERSITY
PULLMAN, WA 99163

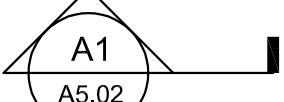


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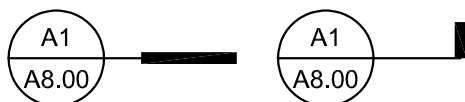
BUILDING SECTION



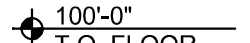
WALL SECTION



DETAIL



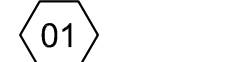
ELEVATION MARK



REVISIONS

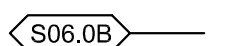


WINDOW TYPES



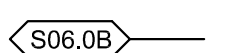
SEE SHEET A6.00

FLOOR & WALL ASSEMBLIES



SEE SHEET A8.00

ROOF ASSEMBLIES

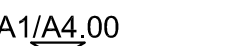


SEE SHEET A8.01

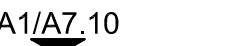
DOOR NUMBER



EXTERIOR ELEVATION



INTERIOR ELEVATION



VICINITY MAP

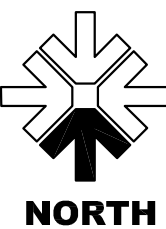


PROJECT LOCATION

1220 NE FAIRWAY RD.

PROJECT LOCATION

1220 NE FAIRWAY RD.



GENERAL NOTES

CONTRACTOR TO VERIFY WITH UTILITIES EXACT LOCATION OF EXISTING UNDERGROUND SERVICES. CONTACT CALL BEFORE YOU DIG AT: 811

1. CONTRACTOR SHALL COORDINATE EFFORTS FOR DESIGN BUILD MECHANICAL, ELECTRICAL, AND PLUMBING SYSTEMS. ALL WORK SHALL COMPLY WITH THE REQUIREMENTS OF THESE DRAWINGS, THE SPECIFICATIONS, AND THE CURRENT EDITIONS OF THE IBC, UMC, UPC, NEC AND THE WASHINGTON STATE ENERGY CODE, AND ALL CITY OF PULLMAN REGULATIONS. CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL REQUIRED PERMITS AND PROVIDING ALL REQUIRED DOCUMENTATION FOR ISSUANCE OF PERMITS, INCLUDING ANY AND ALL ENGINEERING, STAMPED AND SIGNED BY REGISTERED PROFESSIONAL ENGINEERS IN THE STATE OF WASHINGTON APPROPRIATE TO THE DISCIPLINE.

2. THE CONTRACTOR AGREES TO ASSUME SAFE AND COMPLETE RESPONSIBILITY FOR THE JOB SITE DURING THE COURSE OF THE PROJECT, INCLUDING THE SAFETY OF ALL PERSONS AND PROPERTY, THAT THIS REQUIREMENT SHALL APPLY CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS, AND THAT THE CONTRACTOR SHALL DEFEND, INDEMNIFY AND HOLD THE ARCHITECT AND OWNER HARMLESS FROM ANY AND ALL LIABILITY, REAL OR ALLEGED, IN CONNECTION WITH THE PERFORMANCE OF WORK ON THIS PROJECT, EXCEPTING FOR LIABILITY ARISING FROM THE SOLE NEGLIGENCE OF THE OWNER OR ARCHITECT, OR THIRD PARTY IN VIOLATION OF THE LAW OR IN TRESPASS. THE CONTRACTOR SHALL PRACTICE SAFETY AT ALL TIMES AND SHALL FURNISH, ERECT, AND MAINTAIN SUCH FENCES, BARRICADES, LIGHTS, AND SIGNS NECESSARY TO GIVE ADEQUATE PROTECTION TO THE PUBLIC AT ALL TIMES. THE ARCHITECT MAKES NO WARRANTY THAT THE CONSTRUCTION-PHASE FENCES, ETC., ARE ADEQUATE TO ENSURE SAFETY DURING CONSTRUCTION OPERATIONS.

3. PLAN INTENT: THESE DRAWINGS REPRESENT EXISTING CONDITIONS (CONTRACTOR TO VERIFY ALL ACTUAL CONDITIONS) AND THE FINISHED FACILITIES AND UNLESS OTHERWISE INDICATED, THEY DO NOT SHOW THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PRODUCE THE FINISHED FACILITIES AND TO PROTECT THE WORK AND PROPERTY, THE SAFETY OF THE WORKERS, AND OTHER PERSONS DURING CONSTRUCTION. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE ABOVE MEASURES AND VISITS TO THE SITE BY THE ARCHITECT SHALL NOT INCLUDE OBSERVATION OF SAFETY MEASURES.

4. FIELD VERIFY ALL BUILDING POINTS OF CONNECTIONS AND FLOOR TO FLOOR HEIGHTS. NEW BUILDING TO MATCH FLOOR HEIGHTS OF EXISTING BUILDING.

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DETAIL 1 C3

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REFLECTED CEILING PLAN A3.50
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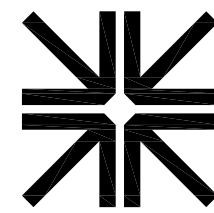
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DESIGN WEST
ARCHITECTS P.A.
• MERIDIAN, IDAHO • PULLMAN, WASHINGTON •
• KENNEWICK, WASHINGTON • OREGON •



DRAWN BY
SG

CHECKED BY
NW

JOB NUMBER
22103

REVISIONS

Sam Gregg 1/26/2024 4:26:57 PM

WASHINGTON UNIVERSITY GOLF PRACTICE FACILITY - ADDITION
Building Code Summary

Authorities having Jurisdiction:

City of Pullman Jeromy Moore, Building Official
190 SE Crestview Street
Pullman, WA 99163 jeromy.moore@pullman-wa.gov

Building Code:

Applicable Requirements:

International Building Code 2018 Edition, with WAC amendments
International Existing Building Code 2018 Edition, with WAC amendments
Washington State Energy Code - 2018 Edition,
ANSI A117.1-2009 - Accessible and Usable Buildings and Facilities

2018 Uniform Plumbing Code
2020 National Electric Code
2018 International Mechanical Code

Existing Building: 3,556 square feet main level practice facility

New Addition: 375 square feet entry, storage room, restroom

Construction type: V- B (existing construction)
V- B (new construction)

Occupancy class: A-3 (existing and new construction)

Commentary ONE- IEBC Compliance:

The work of this project is considered under the requirements of the International Existing Building Code, 2018 edition (IEBC). The project is defined as an addition by the IEBC. The new construction work shall comply with Chapter 11 of the IEBC and the applicable portions of the 2018 IBC..

In summary:

- Section 1101.3 - The minor amount of work in the adjacent existing space shall comply with Chapter 6, and the prescriptive requirements of the IEBC.
- Section 1102 - The addition is within the allowable heights, areas, and allowance for non-fire protected building.
- Section 1103 - The addition is structurally independent of the existing structure.
- Sections 1104 through 1106 - are not applicable to this project.
- Section 1107 - The addition on this project shall comply with the energy code.

Commentary TWO - IBC Compliance Chapter 5

The existing building and new addition result in the aggregate building area and the heights of the building are within the limitations specified in Sections 504 and 506.

In summary:

- Section 504.3 - The project is less than 40' in height.
- Section 504.4 - The project is one story.
- Section 506.2 - Allowable Area Factor : Aa = 6,000 allowable where total project area is 3,931 SF.

Commentary THREE- Energy Code Compliance:

All new construction shall comply with energy code requirements by way of the prescriptive requirements defined by WSEC. Reference WSEC compliance worksheet below.



12/19/23, 10:49 AM wsenergocodes.com/print_project_summary_form.php?sv=Y29Y16R6GZ2K3B0P9N7Y19edWwaWV9p2SkaX8p2ZCU3Q2BkX2PhZG3daWVWuJh0nq1xY2M2vvaHRFY2haZG5h...

ENVELOPE COMPLIANCE SUMMARY

2018 WSEC Compliance Form for Commercial Buildings Including Group R2, R3 & R4 and 3 stories and all R1

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Project Title	WSU Golf Team Practice Facility Upgrade - 2018 WSEC	For Building Department Use:	Date: Dec 19, 2023
Project Address	1200 N. Fortney St., Pullman, WA 99163		
Applicant Name	WSU		
Applicant Phone	509-337-5111		
Applicant Email	gong@wsu.edu		

For questions about this report, contact WSU's Commercial Technical Support at 509-339-1300 or via email at comtechsupport@wsuenergocodes.com

General Occupancy	All Commercial	General Building Use Type(s)	Entire Assembly, Sports Culture Club	Building Code, Floor Area	2,616
Project Scope	Building Addition	Space Conditioning Category	Fully Conditioned	Project Code, Floor Area	2,616
Envelope Project Description				Compliance Method	1 - General

Envelope Compliance Scope and Method	Scope	Space Conditioning Category	Compliance Method	W/W/WRB per Category	UA Calculation Adjustment	Penetration Alternative	Compliance Verification
	Building Addition	Fully Conditioned	Prescriptive	24.0% (0%)	None selected	No details selected	COMPLIES

See Envelope Details

Project Title: WSU Golf Team Practice Facility Upgrade - 2018 WSEC

Scope & Space Conditioning: BUILDING ADDITION - FULLY CONDITIONED

Compliance Verification: COMPLIES

Envelope Details Summary: 24.0% Envelope Insulation Alternative: No details selected

Roof/Ceiling: Location in Documents: Assembly ID: Assembly Location: Ceiling: Insulation R-Value: 2nd Layer (MR Roof): U-Factor: Net Area (SF):

Walls: Location in Documents: Assembly ID: Assembly Location: Ceiling: Insulation R-Value: 2nd Layer (MR Roof): U-Factor: Net Area (SF):

Floors: Location in Documents: Assembly ID: Assembly Location: Ceiling: Insulation R-Value: 2nd Layer (MR Roof): U-Factor: Net Area (SF):

Glazing: Location in Documents: Assembly ID: Assembly Location: Ceiling: Insulation R-Value: 2nd Layer (MR Roof): U-Factor: Net Area (SF):

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JOB NUMBER 22103

REVISIONS

WSU GOLF COURSE

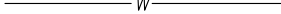
INDOOR PRACTICE FACILITY UTILITIES

WASHINGTON STATE UNIVERSITY
Department of Facilities Operations Pullman, Washington

WASHINGTON STATE UNIVERSITY
Department of Facilities Operations Pullman, Washington

WASHINGTON STATE UNIVERSITY
Department of Facilities Operations Pullman, Washington

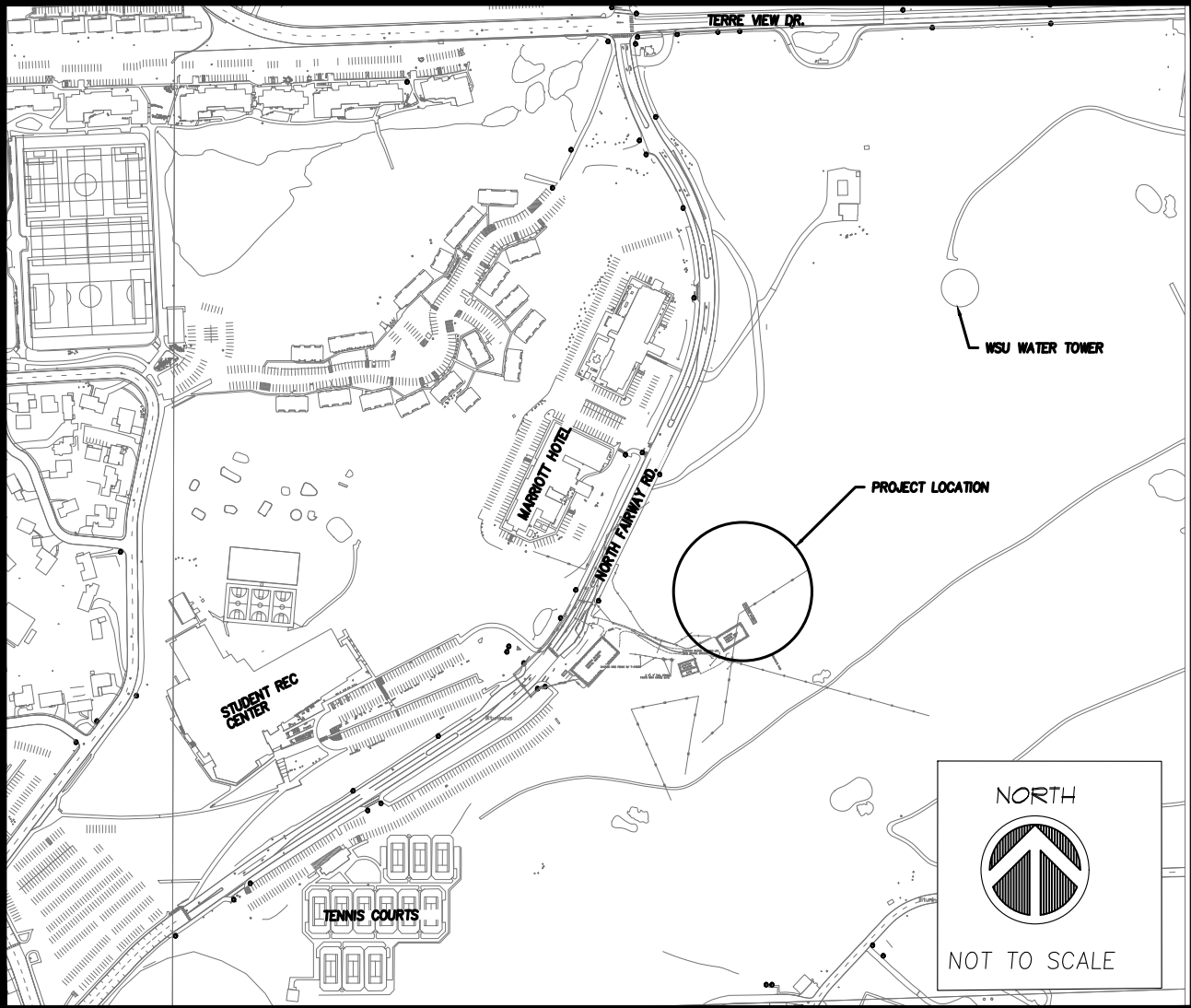
LEGEND of SYMBOLS (EXISTING AND PROPOSED)

EXISTING		PROPOSED	
 2535	MAJOR CONTOUR	 SAWCUT	CONTOUR
 2531	MINOR CONTOUR	 2560	CURB
 SS	SANITARY SEWER		STORM LINE
 SD	STORM		STORM MANHOLE
 W	WATER		CATCH BASIN
 AP	AERIAL POWER		ASPHALT
 BP	BURIED POWER		GRAVEL
 GAS	BURIED GAS		SIGN
 COMM	COMMUNICATIONS		SPOT EL.
 CURB ONLY			DRAINAGE FLOW ARROW
 CURB AND GUTTER			BMP DESIGNATION PER STORMWATER MANAGEMENT MANUAL FOR EASTERN WASHINGTON
 EDGE OF ASPHALT			
 FENCE			
 DRAINAGE DITCH			
 CONCRETE			
 BUILDING			
 GRAVEL SURFACE			
	STORM MANHOLE		POWER/TELEPHONE POLE
	CATCH BASIN		POLE ANCHOR
 RD	ROOF DRAIN	 P	POWER TRANSFORMER
	FLOOR DRAIN		POWER TRANSFORMER
	SANITARY SEWER MANHOLE	 E	ELECTRIC METER
 CO	CLEAN OUT	 J	JUNCTION BOX
 V	IRRIG. CONTROL VALVE	 G	GAS METER
 W	WATER METER		BOLLARD
 H	HOSE BIB/YARD HYDRANT	 X	HANDICAP RAMP
 W	WATER MANHOLE		LIGHT POLE
 V	WATER VALVE		SIGN W/BLADES
 F	FIRE HYDRANT		MONUMENT SIGN
 F	FIRE DEPARTMENT CONNECTION		CONIFEROUS TREE W/TRUNK SIZE
 E	END CAP		DECIDUOUS TREE W/TRUNK SIZE
 PIV	POST INDICATOR VALVE		SHRUB
			BENCH MARK

INDEX of SHEETS

INDEX of SHEETS

LOCATOR MAP - W.S.U. CAMPUS



DEMOLITION NOTES

1. CONTRACTOR TO REVIEW PROJECT LIMITS FOR QUANTITY AND TYPE OF STRUCTURES, GROUND COVER AND DEBRIS AT THE TIME OF BIDDING AND SHALL INCORPORATE REMOVALS/DISPOSAL IN THEIR WORK.
2. CONTRACTOR SHALL OBTAIN ALL CONSTRUCTION APPROVALS/PERMITS FOR THE PROJECT SITE FROM WASHINGTON STATE UNIVERSITY, AND OTHER GOVERNING AGENCIES PRIOR TO ANY CONSTRUCTION ACTIVITY.
3. CONTRACTOR SHALL BE RESPONSIBLE TO REMOVE ANY AND ALL ITEMS NOT OTHERWISE LISTED HEREIN THAT CONFLICT WITH THE CONSTRUCTION OF THE PROJECT. CONTRACTOR SHALL CONTACT PROJECT MANAGER IMMEDIATELY UPON ENCOUNTERING ITEM TO DETERMINE ANY ITEMS NOT SHOWN ON THE PLANS THAT MUST BE REMOVED. FAILURE TO DO SO DOES NOT RELIEVE CONTRACTOR OF COST RESPONSIBILITY FOR REMOVING ITEMS REQUIRED.
4. LOCATION OF ALL UNDERGROUND UTILITIES SHOWN ARE APPROXIMATE AND CONTRACTOR SHALL VERIFY CONNECTION POINTS WITH EXISTING UTILITIES. CONTRACTOR IS RESPONSIBLE FOR ANY DAMAGE CAUSED TO EXISTING UTILITIES AND UTILITY STRUCTURES THAT ARE TO REMAIN. RECORD LINE CAPPING LOCATIONS AND ACCURATE UTILITY LOCATIONS (HORIZONTAL & VERTICAL) WITH DIMENSION STRINGS TO PERMANENT LANDMARKS ON AS-BUILT PLAN FOR WSU.
5. ALL DEMOLISHED ASPHALTIC CONCRETE AND PORTLAND CEMENT CONCRETE SHALL BE REMOVED FROM SITE AND DISPOSED OF IN A LEGAL MANNER PROVIDED BY WSU FACILITIES.
6. THE CONTRACTOR SHALL TAKE EFFECTIVE ACTION TO PREVENT THE FORMATION OF ANY AIRBORNE DUST NUISANCE AND SHALL BE RESPONSIBLE FOR ANY DAMAGE RESULTING FROM FAILURE TO FOLLOW WSU AND CITY OF PULLMAN GUIDELINES. THE CONTRACTOR MUST PUT INTO PLACE MEANS CONFORMING TO BEST MANAGEMENT PRACTICES TO PREVENT SURFACE WATER CONTAMINATION. STORM DRAINS THAT MAY CARRY SEDIMENT AND CONTAMINANTS MUST BE PROTECTED.
7. CONTRACTOR SHALL SUBMIT A WRITTEN HAIL ROAD PLAN FOR THE PROJECT SITE TO PROJECT MANAGER AND OBTAIN APPROVAL FROM THEM PRIOR TO USE. VERIFY ALL OVERHEAD CLEARANCES PRIOR TO COMMENCING WORK.
8. CONTRACTOR SHALL SUBMIT A WRITTEN TRAFFIC CONTROL PLAN FOR THE PROJECT SITE TO PROJECT MANAGER FOR APPROVAL PRIOR TO CONSTRUCTION. THE PLAN SHALL BE IN CONFORMANCE WITH THE CITY OF PULLMAN AND MUTCD (MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES) STANDARDS AND SHALL BE MAINTAINED AND AMENDED AS CONDITIONS CHANGE THROUGHOUT CONSTRUCTION.
9. CONTRACTOR SHALL PROVIDE ALL SIGNS AND BARRICADES REQUIRED TO CLOSE PARKING AND ROADWAY AREAS, REDIRECT TRAFFIC AND PEDESTRIANS, AND PROVIDE ADVANCE WARNING AND INSURE PUBLIC SAFETY. TEMPORARY SIGNS AND BARRICADES SHALL BE PER WSDOT STANDARD SPECIFICATIONS. NO HAND PAINTED SIGNS WILL BE ALLOWED.
10. CONTRACTOR SHALL PREVENT THE TRACKING OF DIRT/SILT FROM THE WORK AREA ONTO ADJACENT ROADWAYS AND PAVED SURFACES, IF REQUIRED. CONTRACTOR SHALL SWEEP EXISTING PAVED SURFACES OF DIRT/SILT PROMPTLY UPON CONTAMINATION OR AS DIRECTED BY WSU.
11. IF AN UNDERGROUND STORAGE TANK OR UNDERGROUND PIPING CONTAINING OR COMPOSED OF ASBESTOS CONTAINING MATERIAL IS ENCOUNTERED, THE CONTRACTOR SHALL IMMEDIATELY CEASE WORK AND REPORT TO THE OWNER'S PROJECT TEAM.
12. THE UNDERGROUND UTILITIES SHOWN HAVE BEEN LOCATED AS ACCURATELY AS POSSIBLE FROM FIELD SURVEY INFORMATION AND EXISTING DRAWINGS, TAYLOR ENGINEERING, INC. MAKES NO GUARANTEES THAT THE UNDERGROUND UTILITIES SHOWN COMPRISE ALL SUCH UTILITIES IN THE AREA, EITHER IN SERVICE OR ABANDONED. FURTHER, WE DO NOT WARRANT THAT THE UNDERGROUND UTILITIES SHOWN ARE IN EXACT LOCATION INDICATED, ALTHOUGH WE DO CERTIFY THAT THEY ARE LOCATED AS ACCURATELY AS POSSIBLE FROM INFORMATION AVAILABLE.

GENERAL NOTES

1. THE CONTRACTOR SHALL FIELD VERIFY ALL EXISTING CONDITIONS PRIOR TO EXECUTING ANY WORK AND SHALL NOTIFY THE DEPARTMENT OF FACILITIES OPERATIONS IN WRITING OF ANYTHING DIFFERING FROM THAT SHOWN ON THE DRAWINGS. FAILURE TO DO SO WILL CONSTITUTE THE CONTRACTOR'S ACCEPTANCE OF THE WORK AS SHOWN.
2. ALL WORK SHALL COMPLY WITH ALL CODES, ORDINANCES AND REQUIREMENTS.
3. DO NOT SCALE DRAWINGS; WRITTEN DIMENSIONS SHALL PREVAIL.
4. THE CONTRACTOR SHALL OBTAIN ALL PERMITS REQUIRED PRIOR TO ANY PHASE OF CONSTRUCTION. FEES AND ANY RELATED COSTS ARE THE RESPONSIBILITY OF THE CONTRACTOR.
5. EXISTING BUILDINGS, UTILITIES, ETC. OR PORTIONS THEREOF, WHICH ARE DAMAGED BY CONSTRUCTION OPERATIONS SHALL BE REPAIRED OR REPLACED TO THEIR ORIGINAL CONDITION, UNLESS OTHERWISE NOTED TO BE REMODELED PER THE CONTRACT DOCUMENTS.
6. THE CONTRACTOR MUST CONTACT "CALL BEFORE YOU DIG," AT 1-(800) 424-5555 AND/OR ALL APPROPRIATE UTILITY COMPANIES FOR BURIED PIPES OR CABLES, PRIOR TO ANY EXCAVATION.
7. EXACT LOCATIONS OF EXISTING UTILITIES ARE TO BE VERIFIED BY THE CONTRACTOR.
8. THE DUTIES OF THE OWNER/WASHINGTON STATE UNIVERSITY, ITS ENGINEER, AND ITS DESIGNATED CONSULTING ENGINEER DOES NOT INCLUDE DESIGN AND CONSTRUCTION REVIEW SERVICES RELATING TO THE CONTRACTORS SAFETY PRECAUTIONS OR TO THE MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES REQUIRED FOR THE CONTRACTOR TO PERFORM HIS WORK.
9. FOLLOWING IS A LIST OF TELEPHONE NUMBERS THAT MAY NEED TO BE CONTACTED DURING THE CONSTRUCTION PHASE OF THE PROJECT:

WSU DEPARTMENT OF FACILITIES OPERATIONS (UTILITY INFORMATION)	(509) 335-9000
AVISTA UTILITIES (GAS & POWER)	(509) 336-9038
CALL BEFORE YOU DIG	1-(800) 424-5555

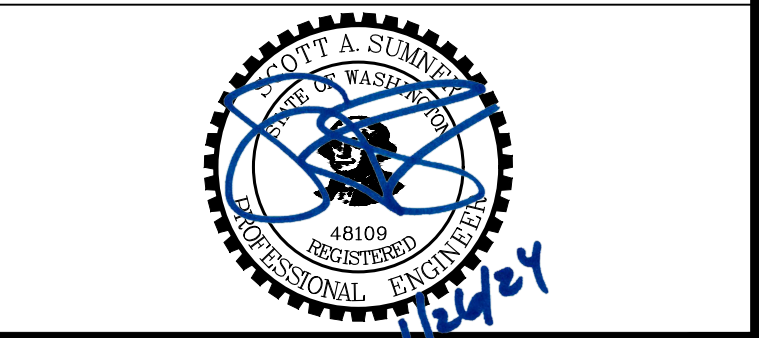
DESIGN FIRM:

SYNTIER
Engineering, Inc.

405 SE Brelsford Dr., Suite C
Pullman, WA 99163
www.SynTierEngr.com 509.339.6187

CONSULTANTS:

PROVISIONAL STAMP:



WASHINGTON STATE
UNIVERSITY

Facilities Services PH 509-335-5571
2425 E. Grimes Way FAX 509-335-9304
Pullman, Wa. 99164-1150

**WSU GOLF INDOOR
PRACTICE FACILITY
NEW UTILITIES**

WSU-01

**WSU GOLF INDOOR
PRACTICE FACILITY
NEW UTILITIES**

WSU-01

	1/26/24	REVISED DRAWINGS
MARK	DATE	DESCRIPTION

CAD DWG FILE: WSU01-C1-COVERSHEET.dwg

DESIGN BY: SAS

DRAWN BY: SAS	CHECKED BY: SAS
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DRAWN BY: SAS	CHECKED BY: SAS
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PROJECT No. WSU-01	FILE No.365B-G-003
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PROJECT No. WSU-01	FILE No.365B-G-003
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SHEET TITLE

COVER SHEET

C1
SHEET 1 OF 3 SHEETS

SHEET 1 OF 3 SHEETS

SHEET 1 OF 3 SHEETS

SHEET 1 OF 3 SHEETS

SHEET 1 OF 3 SHEETS

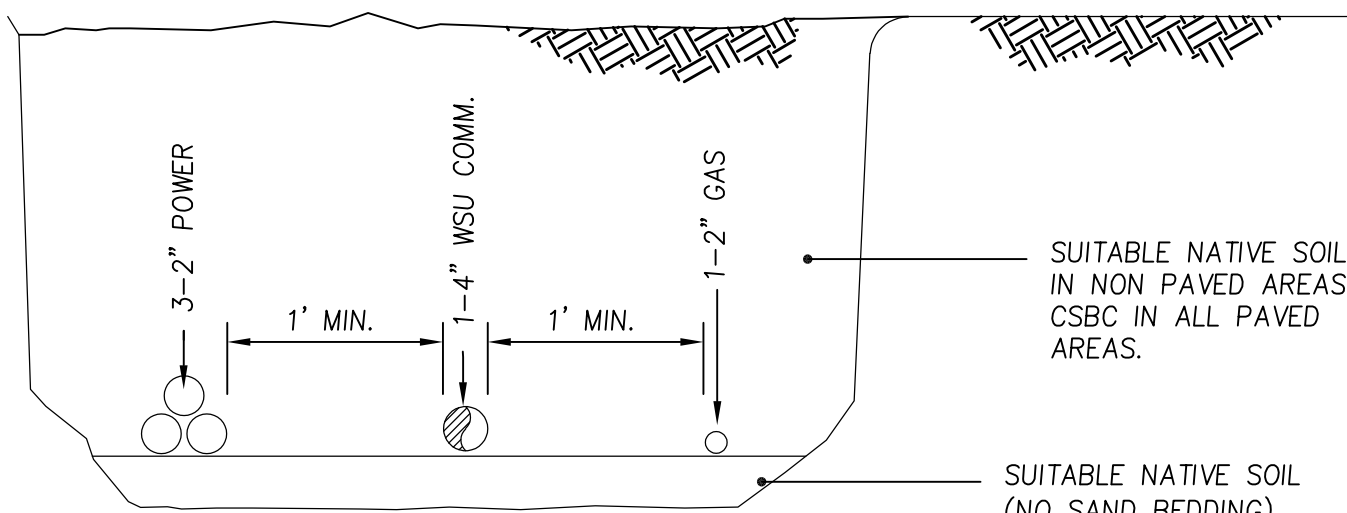
FILE: P:\WSU-01 (WSU Golf Indoor Practice Facility)\WSU01-DWG\Sheets\WSU01-C2-UTILITY PLAN.dwg PLOT DATE: OCTOBER 12, 2021 ORIGINAL PAPER SIZE: 22" x 34"

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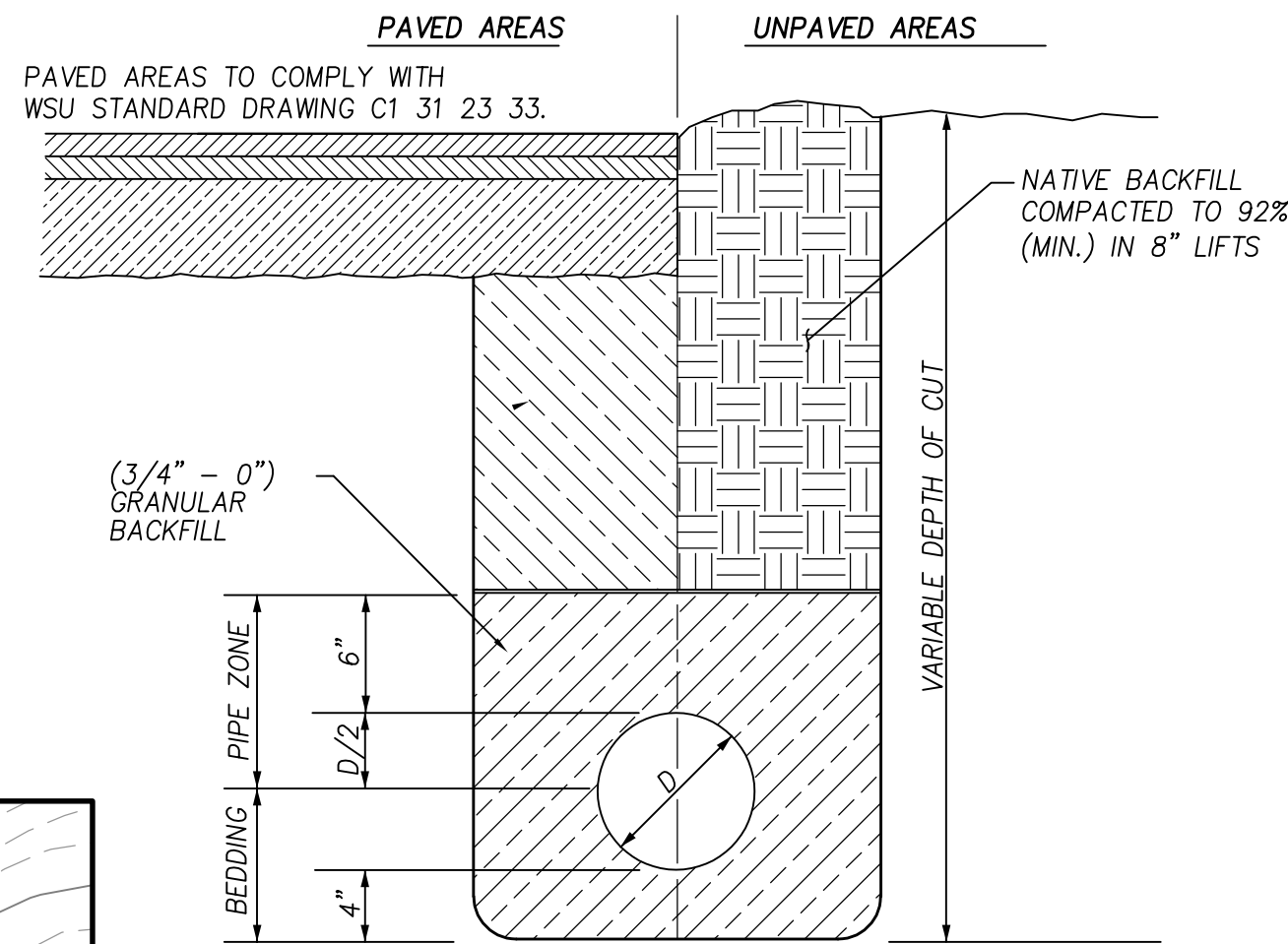
DEMOLITION/REMOVAL LEGEND:

- (P) CONTRACTOR SHALL PROTECT AND MAINTAIN IN PLACE.
(R) CONTRACTOR SHALL REMOVE AND DISPOSE OF LEGALLY OFF-SITE

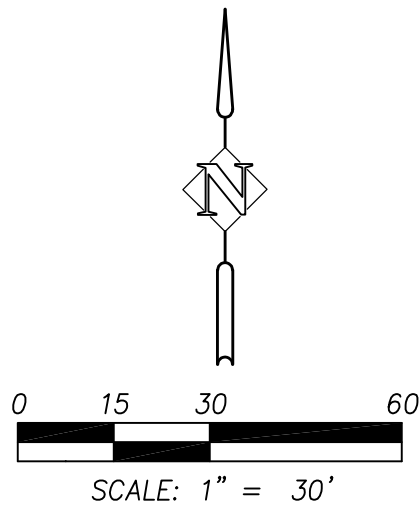
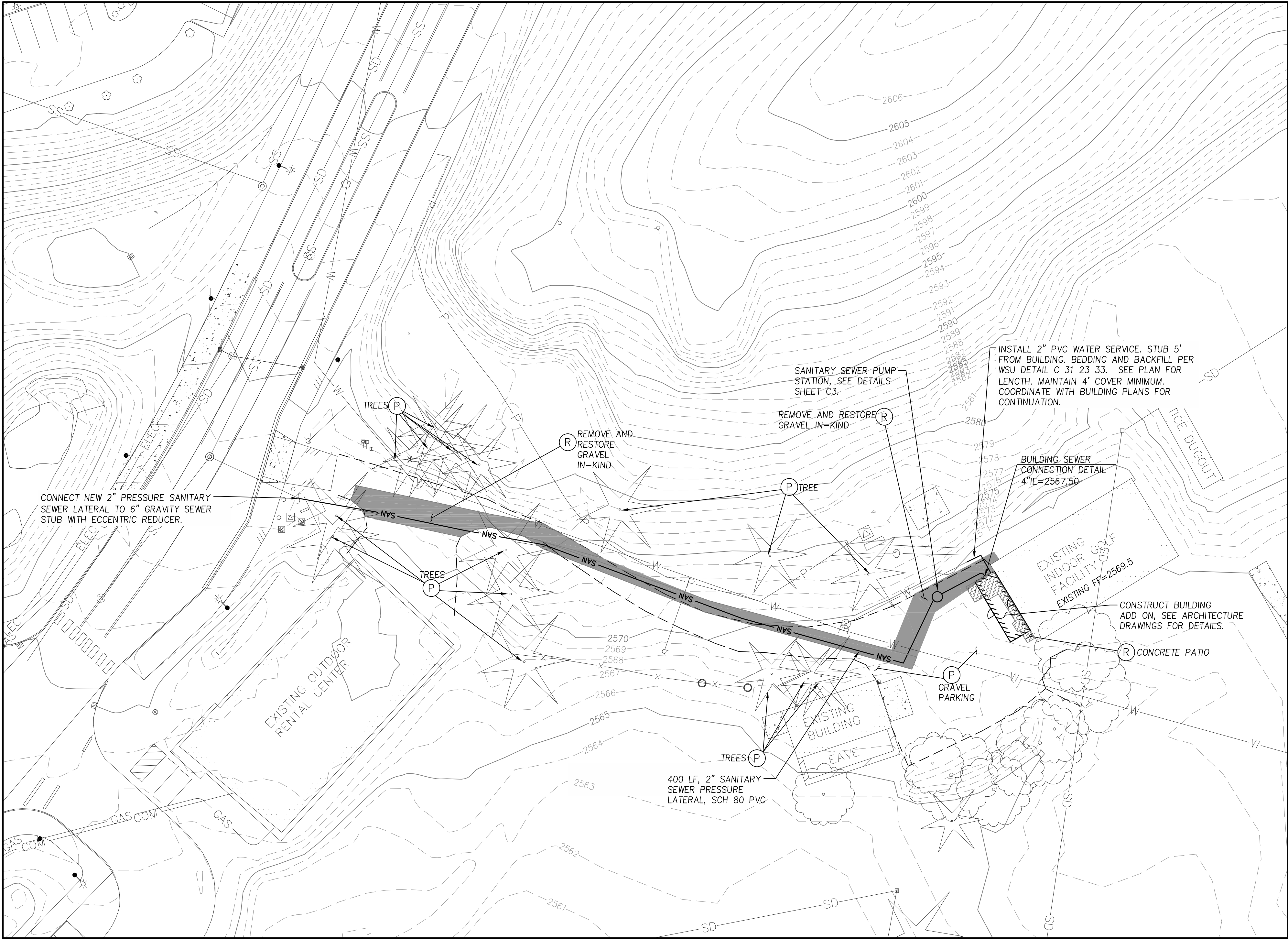


1. IN AREAS WHERE FRANCHISE UTILITY COINCIDES WITH DRAINAGE DITCH, 36" COVER BEGINS AT BOTTOM OF DITCH

FRANCHISE COMMON TRENCH
N.T.S.



PIPE BEDDING AND BACKFILL DETAIL
N.T.S.



DESIGN FIRM:

SYNTIER
Engineering, Inc.
405 SE Brelsford Dr., Suite C
Pullman, WA 99163
www.SynTierEngr.com 509.339.6187

CONSULTANTS:

PROVISIONAL STAMP:



WASHINGTON STATE
UNIVERSITY

Facilities Services PH 509-335-5571
2425 E. Grimes Way FAX 509-335-9304
Pullman, Wa. 99164-1150

**WSU GOLF INDOOR
PRACTICE FACILITY
NEW UTILITIES**

WSU-01

	1/26/24	REVISED DRAWINGS
MARK	DATE	DESCRIPTION

CAD DWG FILE: WSU01-C2-UTILITY PLAN.dwg

DESIGN BY: SAS

DRAWN BY: SAS

PROJECT No. WSU-01

CHECKED BY: SAS
FILE No.365B-G-003

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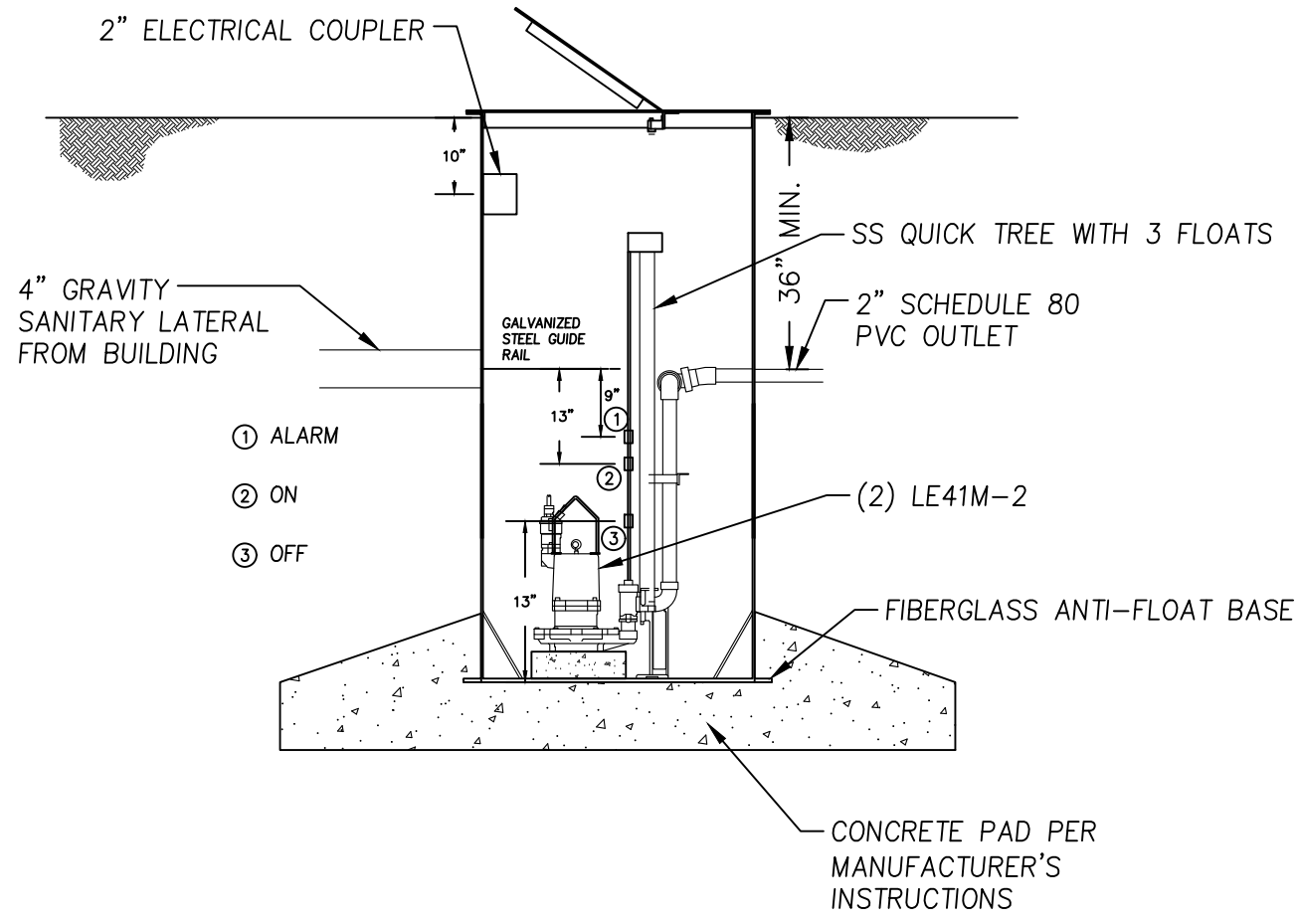
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UTILITY PLAN

C2

SHEET 2 OF 3 SHEETS

FILE: P:\WSU-01 (WSU Golf Indoor Practice Facility)\WSU01-DWG5\Sheets\WSU01-C3-DETAILS.dwg PLOT DATE: OCTOBER 12, 2021 ORIGINAL PAPER SIZE: 22" x 34"



SANITARY SEWER LIFT STATION/WETWELL
N.T.S.

PUMP SCHEDULE										
PUMP			MOTOR		ELECTRICAL				BASIS OF DESIGN	
TYPE	FLOW (GPM)	TOTAL HEAD (FT)	SPEED	HP	V	HZ	PHASE	FLA	MANUFACTURER	MODEL
SUBMERSIBLE	37	17	1725	4/10	115	60	1	12	LIBERTY	LE41M-2
SUBMERSIBLE	37	17	1725	4/10	115	60	1	12	LIBERTY	LE41M-2

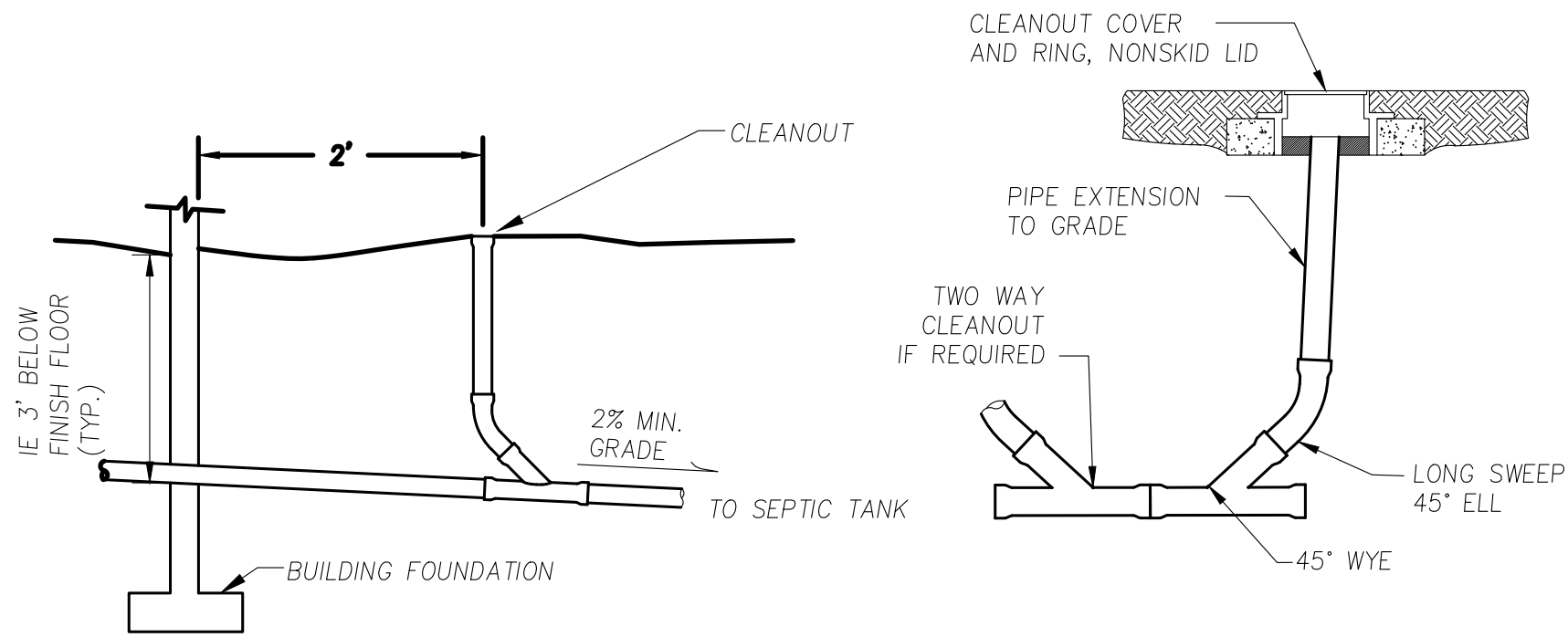
1. DUPLEX ALTERNATING CONTROL PANEL, NEMA 4X, INCLUDING THREE FLOATS, AUX. CONTACTS. INTEGRAL AUDIO/VISUAL HIGH LEVEL ALARM, LIBERTY #AE24L=3

2. DUPLEX FIBERGLASS BASIN AND BLANK COVER, 36" DIAMETER X 120", ANTI-FLOAT RING, 2" DIAMETER SIDE DISCHARGE, LIBERTY EPS70958M

3. TWO GALVANIZED STEEL GUIDE RAIL ASSEMBLIES, LIBERTY #GR22-S

4. 4" DIAMETER FIELD MOUNTED INLET HUB, PVC BALL VALVES, PVC CHECK VALVES, FLOAT BRACKET

5. JUNCTION BOX WITH WATERTIGHT CABLE CONNECTORS AND 2" CONDUIT COUPLING, LIBERTY #JB70.



TYPICAL SEWER CONNECTION

CLEANOUT DETAILS

- NOTES:
- PIPE DIAMETER SHALL BE 4 INCHES OR GREATER. SEE PLAN SHEETS FOR ACCEPTABLE MATERIALS.
 - PIPES STUBBED OUT FOR FUTURE BUILDING CONNECTIONS OR PUBLIC UTILITY CONNECTION SHALL BE PLUGGED WITH A FITTING APPROVED BY THE MANUFACTURER.
 - MAXIMUM DISTANCE BETWEEN CLEANOUTS SHALL BE 100 FEET, THE MAXIMUM AGGREGATE CHANGE IN DIRECTION BETWEEN CLEANOUTS SHALL BE 135°. CONSTRUCT ADDITIONAL CLEANOUTS AS NECESSARY.
 - A WATER SERVICE AND A SEWER SERVICE MAY BE PLACED IN THE SAME TRENCH IF THE WATER SERVICE IS PLACED ON AN UNDISTURBED EARTH SHELF 12 INCHES (MIN) ABOVE THE SIDE SEWER.
 - MINIMUM COVER IN TRAFFIC AREAS IS 3 FEET (2 FEET IN LANDSCAPED AREAS)

BUILDING SEWER CONNECTION DETAILS
N.T.S.

DESIGN FIRM:

SYNTIER
Engineering, Inc.
405 SE Brelsford Dr., Suite C
Pullman, WA 99163
www.SynTierEngr.com 509.339.6187

CONSULTANTS:

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WASHINGTON STATE UNIVERSITY
Facilities Services PH 509-335-5571
2425 E. Grimes Way FAX 509-335-9304
Pullman, Wa. 99164-1150

**WSU GOLF INDOOR PRACTICE FACILITY
NEW UTILITIES**

WSU-01

	1/26/24	REVISED DRAWINGS
MARK	DATE	DESCRIPTION

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FILE No. 365B-G-003

SHEET TITLE

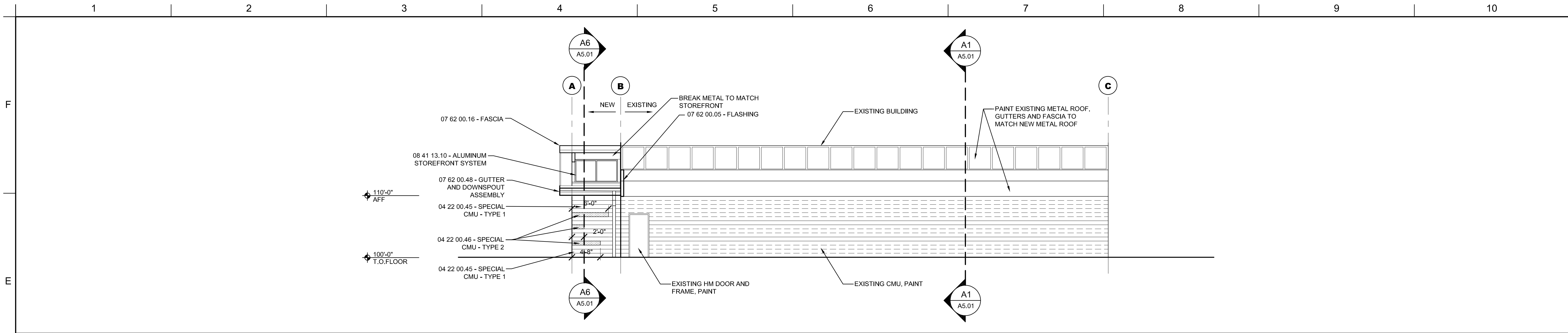
DETAILS

C3
SHEET 3 OF 3 SHEETS

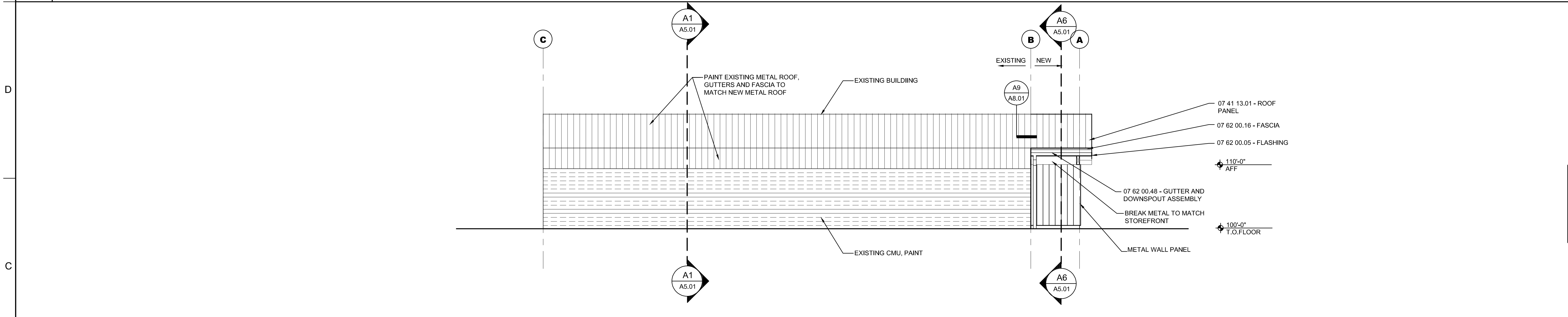
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A3.60

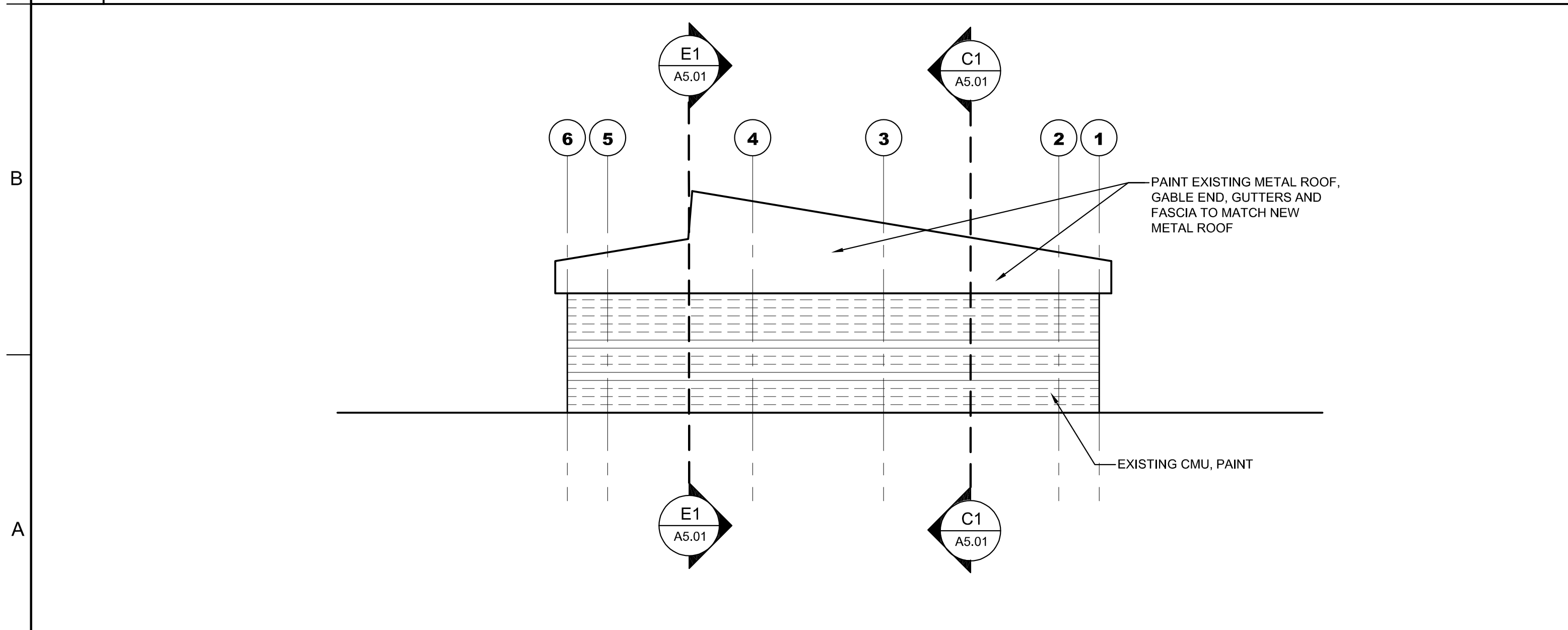
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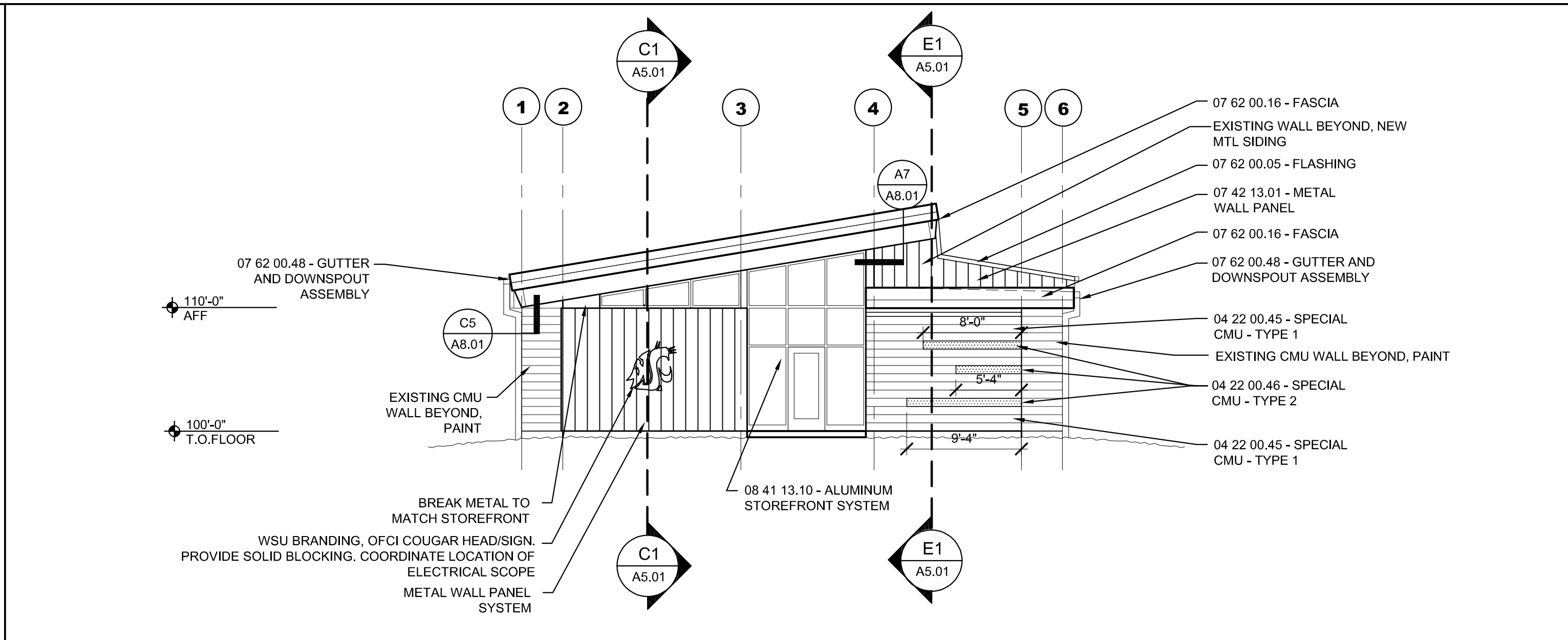
E1 EAST ELEVATION
SCALE: 1/8"=1'



C1 WEST ELEVATION
SCALE: 1/8"=1'



A1 NORTH ELEVATION
SCALE: 1/8"=1'



A6 NORTH ELEVATION
SCALE: 1/8"=1'

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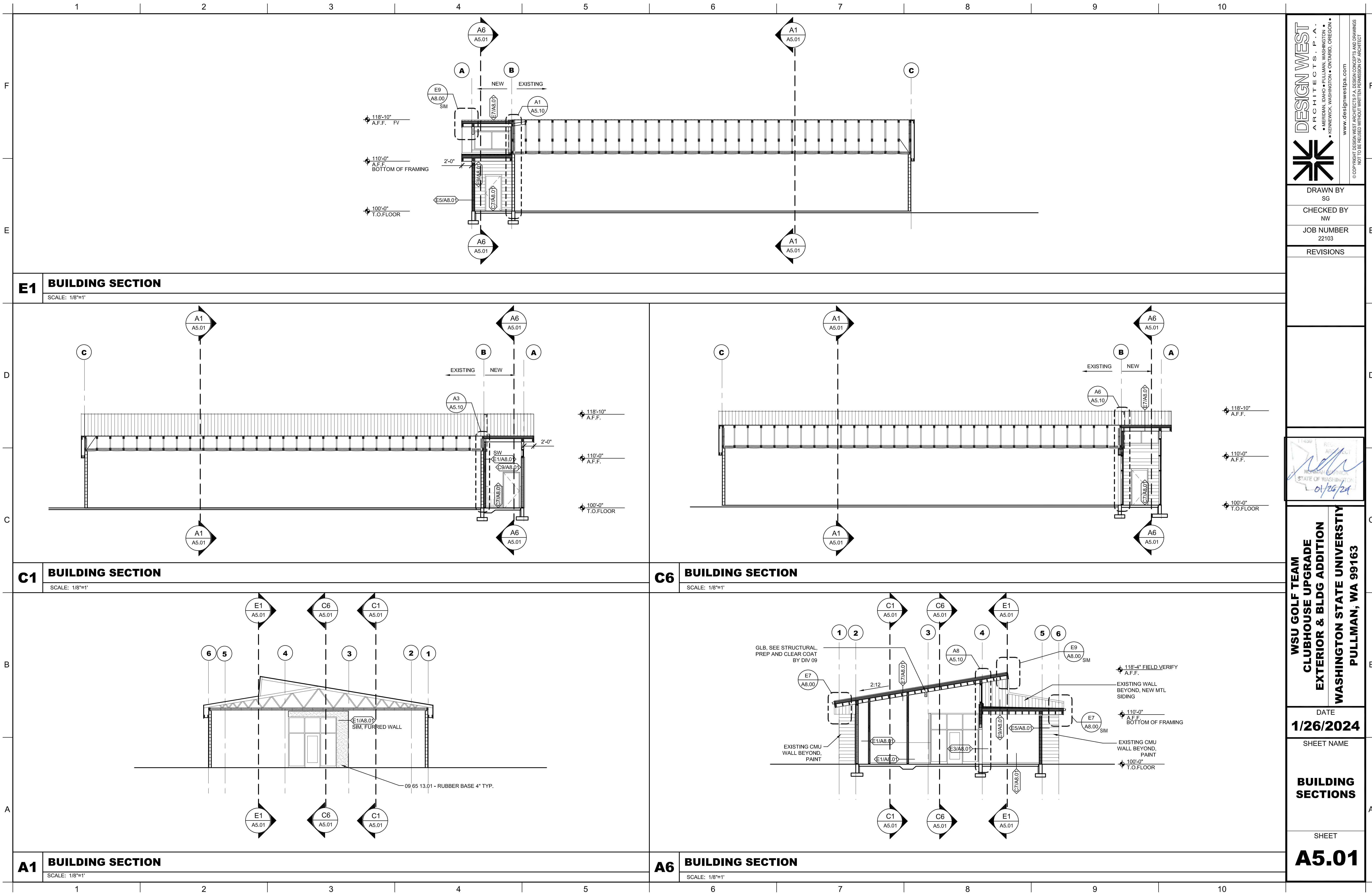
WSU GOLF TEAM
CLUBHOUSE UPGRADE
EXTERIOR & BLDG ADDITION
WASHINGTON STATE UNIVERSITY
PULLMAN, WA 99163

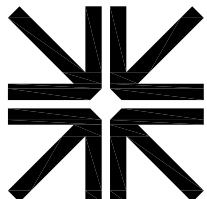
DATE
1/26/2024

SHEET NAME
EXTERIOR ELEVATIONS

SHEET
A5.00

Sam Ganga 1/26/2024 12:43 PM





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JOB NUMBER
22103

REVISIONS

WSU GOLF TEAM
CLUBHOUSE UPGRADE
EXTERIOR & BLDG ADDITION
WASHINGTON STATE UNIVERSITY
PULLMAN, WA 99163

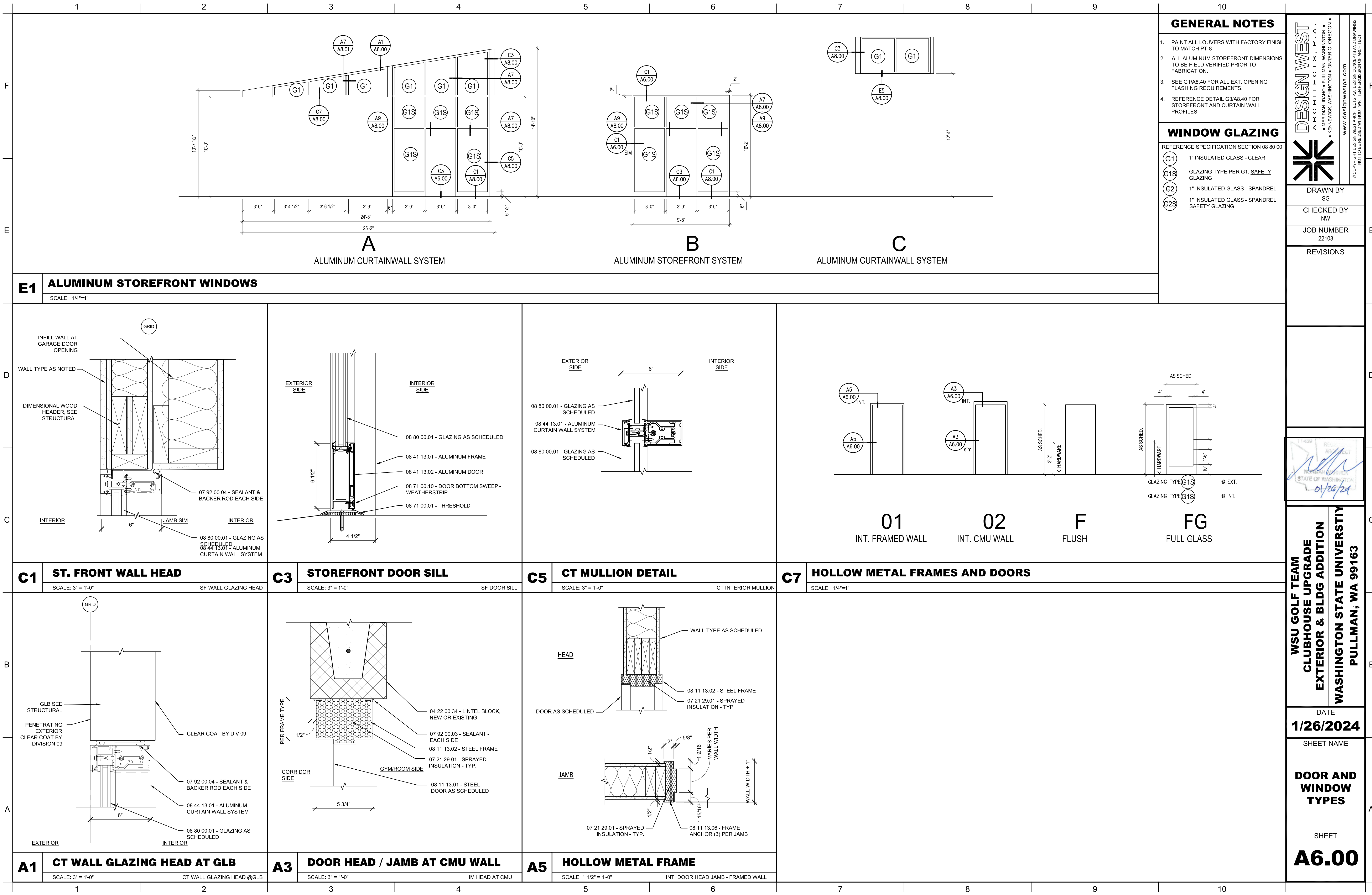
DATE
1/26/2024

SHEET NAME
BUILDING SECTIONS

SHEET
A5.01

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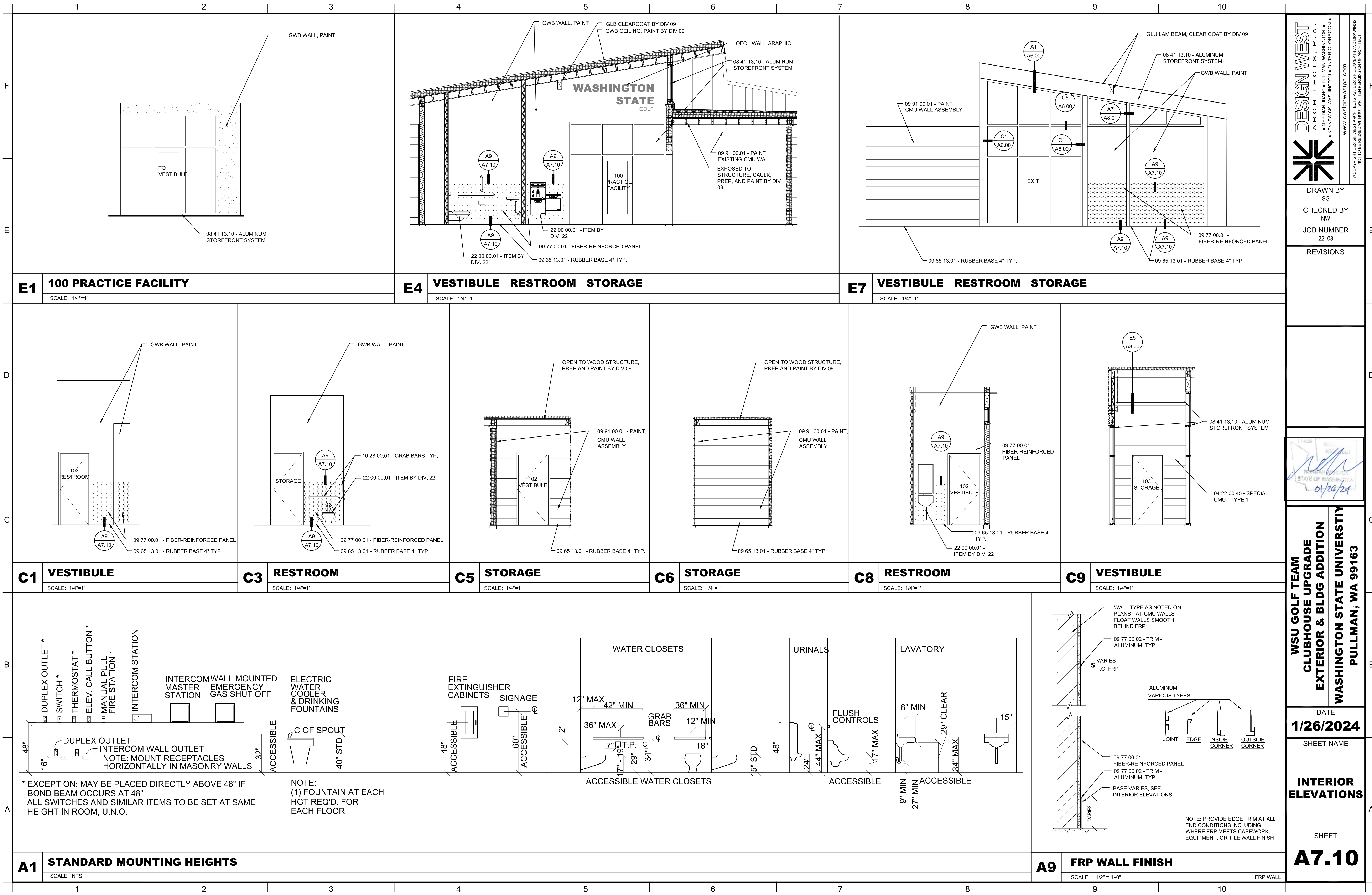
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22103
REVISIONS

WSU GOLF TEAM CLUBHOUSE UPGRADE EXTERIOR & BLDG ADDITION
WASHINGTON STATE UNIVERSITY PULLMAN, WA 99163

DATE
1/26/2024
SHEET NAME
DOOR AND WINDOW TYPES

SHEET
A6.00

Sam Grogg 1/26/2024 1:28:05 PM



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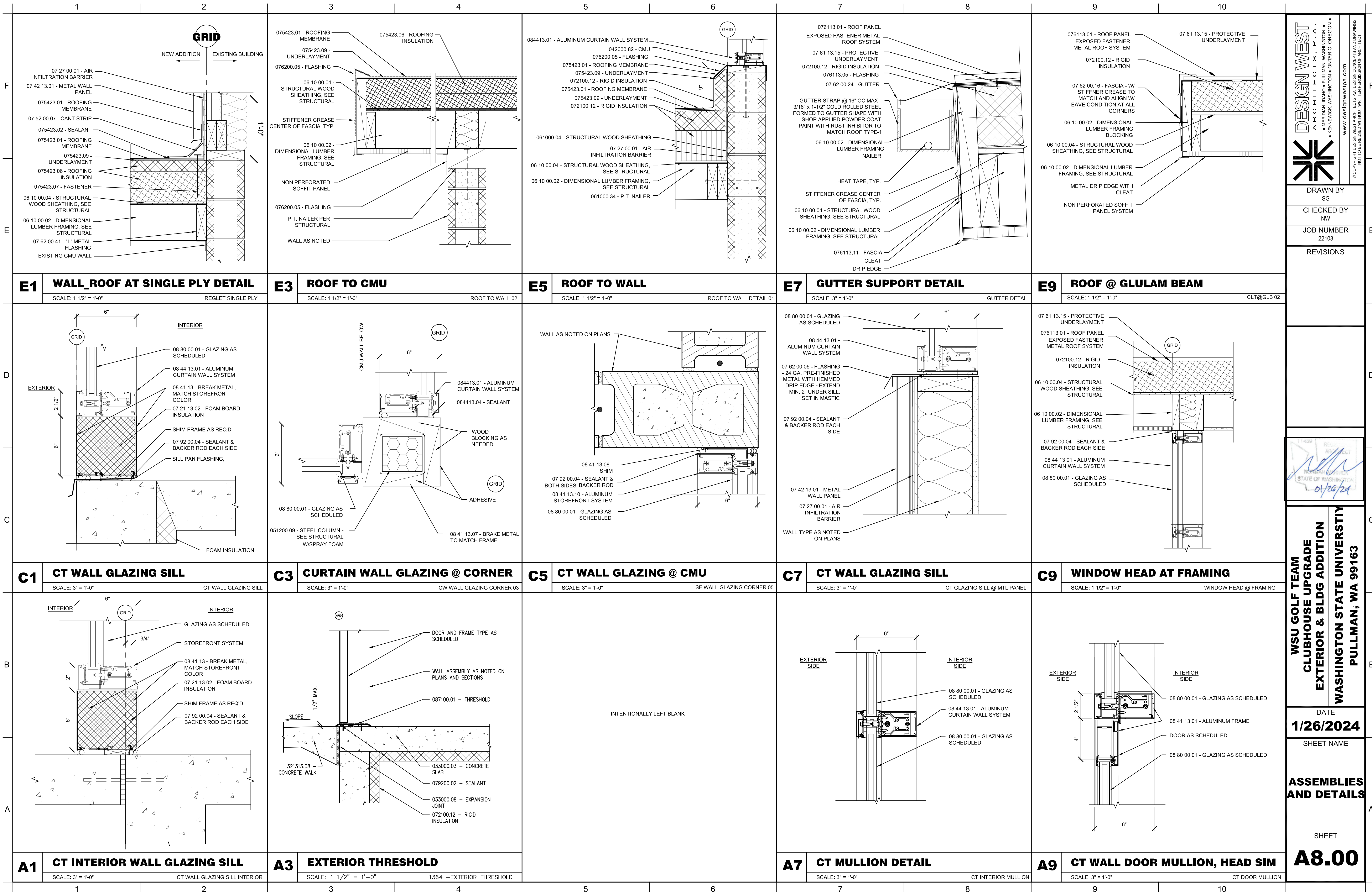
WSU GOLF TEAM
CLUBHOUSE UPGRADE
EXTERIOR & BLDG ADDITION
WASHINGTON STATE UNIVERSITY
PULLMAN, WA 99163

DATE	1/26/2024
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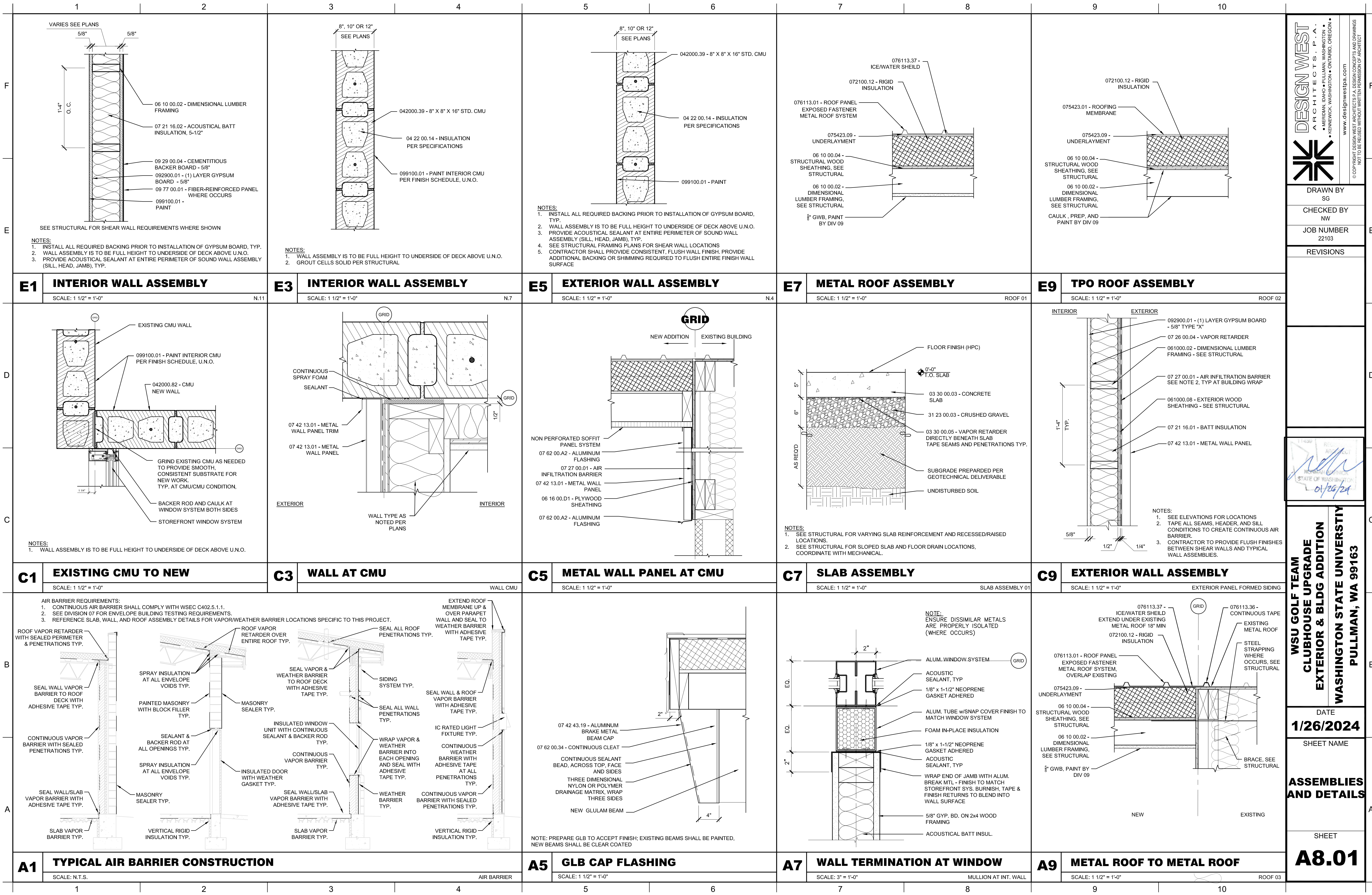
SHEET NAME	INTERIOR ELEVATIONS
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SHEET	A7.10
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	1	2	3	4	5	6	7	8	9	10																															
	GENERAL STRUCTURAL NOTES																																								
F	GENERAL The General Contractor shall verify all dimensions and site conditions before starting work. The Architect and Structural Engineer shall be notified of any discrepancy. Use details marked "Typical" wherever applicable. Changes, omissions or substitutions are not permitted without written approval of the Architect and Structural Engineer. Refer to specifications for further requirements. In the case of conflicts between plans and specifications, the more restrictive requirements shall apply. Any discrepancies on the Contract Documents should be brought to the attention of the Architect and Structural Engineer prior to proceeding with the work. All materials and workmanship shall conform to the 2018 edition of the International Building Code (IBC) and the 2016 edition of "Minimum Design Loads for Buildings and Other Structures" by the American Society of Civil Engineers. Use the latest edition of Referenced Standards as noted in Chapter 35 of the IBC. The Contract structural drawings represent the finished structure. The Contractor is responsible for the means, methods and sequence of construction. The design, adequacy and safety of erection bracing, shoring, temporary supports, etc., is the sole responsibility of the Contractor, and has not been considered by the Structural Engineer. The Contractor is responsible for the stability of the structure prior to the completion of all shear walls, roof and floor diaphragms and finish materials. The Contractor shall provide the necessary bracing to provide stability to the structure prior to the installation of the permanent lateral force resisting system. Observation visits to the site by the Structural Engineer shall not include inspection of temporary bracing, shoring, etc. The Contractor shall coordinate the size and locations of all openings in roofs, floors and walls. Openings shall not penetrate any structural elements such as beams, columns, walls, slabs, etc. unless noted on the drawings or approved by the Structural Engineer. EXISTING CONSTRUCTION Existing structural dimensions and member sizes are for reference only. The contractor shall field verify all dimensions, member sizes, and existing conditions prior to the commencement of new work and modifications to existing construction. Notify the Architect and Structural Engineer of any discrepancies and unsound conditions found. DEFERRED SUBMITTALS Items designed by a Specialty Structural Engineer shall be submitted to the Architect and Structural Engineer for review and shall bear the seal and signature of an engineer registered in the state of Washington. Submittals shall include calculations, design drawings, product data and shop drawings. Design shall be based on information provided on the drawings for gravity, wind and seismic forces unless noted otherwise. The Contractor shall coordinate the seismic restraint of mechanical, plumbing and electrical equipment and the associated piping to the structure. Connections to the structure shall conform to SMACNA standards, be detailed on the contract drawings or be designed by an engineer registered in the state of Washington and submitted for Architect and Structural Engineer review prior to fabrication. SHOP DRAWINGS Shop drawings are to be checked and approved by the General Contractor prior to submitting for Architect and Structural Engineer review. General Contractor is responsible for verification and coordination of dimensions and details for each Subcontractor. Quantities and dimensions are not reviewed by the Structural Engineer and therefore must be verified by the General Contractor. No fabrication shall proceed until shop drawings have been approved. Review of the Specialty Structural Engineer's design submittals and shop drawings is for general compliance with design criteria and compatibility with the design of the primary structure and does not relieve the Specialty Structural Engineer of responsibility for that design. DESIGN LOADS Building Risk Category ----- II Snow Loading Ground Snow Load, P_g ----- 30 PSF Flat Roof Snow Load, P_r ----- 28 PSF + Drifting Snow Exposure Factor, C_e ----- 1.1 Thermal factor, C_t ----- 1.1 Snow Load Importance Factor, I_s ----- 1.0 Wind Loading Basic Wind Speed (3-second gust)----- 110 MPH Wind Exposure ----- C Wind Importance Factor, I_w ----- 1.0 Enclosure Classification ----- Enclosed Seismic Loading - Equivalent Lateral Force Procedure Seismic Importance Factor, I_e ----- 1.0 Spectral Response Accelerations ----- S_s = 0.295 S₁ = 0.107 Site Class ----- D (default) Seismic Design Category ----- C Site Coefficient F_a ----- 1.564 Site Coefficient F_v ----- 2.386 Spectral Response Coefficients ----- S_{DS} = 0.308 S_{D1} = 0.170 Basic Seismic Force-Resisting System --- CLT Shear Panels Response Modification Factor, R ----- 1.5 Seismic Response Coefficient, C_s ----- 0.205 (LRFD) FOUNDATION Assumed allowable soil bearing pressure = 1,500 PSF per IBC Table 1806.2. Bottom of all footings to bear on native, inorganic, undisturbed soil 1'-0" minimum below existing grade. Extend all exterior footings 2'-6" minimum below finished grade. No footing shall bear higher than a 1 vertical to 1.5 horizontal slope above any excavation, existing or planned. Contractor shall provide temporary shoring to prevent movement of walls if backfill is placed before floor system is in place. Site preparation, grading, backfilling, etc. shall be as recommended by the geotechnical report. There shall be 95% compaction (ASTM D1557 Modified Proctor Density) of all backfill soil under slabs on grade. CAST-IN-PLACE CONCRETE Concrete mix proportions in accordance with ACI 301 to meet or exceed the requirements as listed below: <table><tr><th></th><th>Minimum 28 Day Compressive Strength</th><th>Maximum Air Entrainment</th><th>Maximum Size Aggregate</th><th>Maximum Water/Cement Ratio</th></tr><tr><td>All structural conc, UNO</td><td>3000 PSI</td><td>None</td><td>3/4"</td><td></td></tr><tr><td>Footings</td><td>3000 PSI</td><td>None</td><td>1-1/2"</td><td></td></tr><tr><td>Foundation stem walls</td><td>3000 PSI</td><td>5%</td><td>3/4"</td><td></td></tr><tr><td>Interior slab on grade</td><td>4000 PSI</td><td>None</td><td>3/4"</td><td>.50(*)</td></tr><tr><td>Exterior slab on grade</td><td>4500 PSI</td><td>6%</td><td>3/4"</td><td>.45</td></tr></table> Maximum Slump: 3" for slabs and footings; 4" for walls, columns and beams. Slumps given are for un-plasticized concrete. Larger slumps may be allowed through the use of a super plasticizer. Construction to be in accordance with the latest edition of ACI 318, "Part 6: Materials & Durability", "Part 8: Reinforcement" and "Part 9: Construction". Location of construction or pour joints for walls, beams, columns, and structural slabs must be approved by the Structural Engineer if different from that shown on plans. (*) Use a maximum water/cement ratio of 0.45 for interior slabs on grade that could be subject to freeze/thaw cycling while saturated during the construction period.											Minimum 28 Day Compressive Strength	Maximum Air Entrainment	Maximum Size Aggregate	Maximum Water/Cement Ratio	All structural conc, UNO	3000 PSI	None	3/4"		Footings	3000 PSI	None	1-1/2"		Foundation stem walls	3000 PSI	5%	3/4"		Interior slab on grade	4000 PSI	None	3/4"	.50(*)	Exterior slab on grade	4500 PSI	6%	3/4"	.45	
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Interior slab on grade	4000 PSI	None	3/4"	.50(*)																																					
Exterior slab on grade	4500 PSI	6%	3/4"	.45																																					
	CONCRETE SLABS ON GRADE The General Contractor shall control and monitor construction procedures and conditions as required to provide a high quality finished slab with minimal cracks, curling, and surface defects. Procedures and conditions under the control of the General Contractor include, but are not limited to: provisions for ambient weather conditions, curing methods selected, rate of placement, joint arrangement and installation, temperature control, subgrade preparation, and reinforcing placement (where indicated). Placement on Vapor Barrier: Slabs poured directly on vapor barrier are intended to facilitate vapor-emission rates frequently required for floor covering installation. Concrete cure time may be longer than for slabs not in direct contact with vapor barrier; General Contractor to coordinate curing and floor covering installation requirements as required. Slab Finishes: Concrete slabs on grade shall be provided with finish techniques in accordance with the floor classification table of ACI 302.1R. Intended use for interior slabs shall be categorized as either Class 2, Class 4, or Class 5, unless noted otherwise. See architectural drawings for required floor finishes. Slab Joints: Except where specifically indicated, joint spacing and arrangement is to be determined by the General Contractor as part of slab installation procedure. A proposed joint layout plan shall be submitted for architect and engineer approval prior to slab pour. In general, apply the following parameters: Maximum joint spacing, subject to factors above: Typical, UNO: 36 times slab thickness Exposed & polished slabs: 24 times slab thickness Maximum aspect ratio between joints: 1.5 to 1 Avoid re-entrant corners POLISHED OR HONED CONCRETE SLABS ON GRADE The General Contractor shall comply with all provisions noted above, plus additional requirements listed here. Crack control and minimization is imperative at all polished slab areas. Concrete Mix: Fly ash, water vapor reducing admixtures, and air entraining admixtures are not to be included in the mix design. Maximum cement content shall be 517# per cubic yard (5.5 sack mix). Mix design shall provide for maximum curing shrinkage of 0.0003" per inch. Slab Joints: A proposed joint-layout plan shall be submitted for architect and engineer approval prior to slab pour. Reference the architectural drawings for preferred joint locations, and follow joint parameters noted above for standard slabs on grade. MASONRY Design Compressive Strength: CMU masonry f_m = 2000 PSI at 28 days. Concrete Masonry Units: ASTM C90-14, medium weight(115 pcf), running bond. Minimum compressive strength = 2000 PSI. Mortar: ASTM C270, Type S, with a minimum compressive strength of 1800 PSI at 28 days. GROUT: ASTM C476 with a minimum compressive strength of 2000 PSI at 28 days with a maximum aggregate size of 3/8". Fill all cells containing reinforcing with grout in lifts not exceeding 5'-4" in height. Stop each grout lift 1 1/2" below the top course in the lift except at the top of the wall. Fill other cells with grout as indicated on drawings. Unless noted otherwise, lap all bar splices 48 diameters minimum. All reinforcement shall be in place prior to grouting with vertical bars held at top, bottom and 192 diameters maximum on center. Contractor shall provide temporary bracing for masonry walls, as required, until connections to floor and/or roof diaphragms are completed. REINFORCING STEEL Welded Wire Fabric: ASTM A1064, Fy = 65 KSI supplied in flat sheets and lapped a minimum of 12" on all sides. Deformed Welded Wire Fabric: ASTM A1064, Fy = 70 KSI supplied in flat sheets and lapped a minimum of 12" on all sides. Deformed Bars: ASTM A615, Grade 60. Weldable Deformed Bars: ASTM A706, Grade 60 for #4 and larger. Securely tie in place with double annealed 16 gauge iron wire or approved clips. Provide chairs, bolsters, etc. to place reinforcement in the proper positions as indicated on the drawings. Concrete Cover: Unless noted otherwise on these drawings, provide clear cover at reinforcing bars as follows: Concrete cast against soil = 3". Formed concrete against soil = 2". Walls, columns and beams exposed to weather = 2". Walls, columns and beams not exposed to weather = 1 1/2". Slab on grade = 2". Detailing of concrete reinforcement bars and accessories shall conform to the recommendations of ACI 315 "Details and Detailing of Concrete Reinforcement" and ACI SP-66 "Detailing Manual". Placing of reinforcement shall conform to ACI 315R "Manual of Engineering and Placing Drawings for Reinforced Concrete Structures" and CRSI "Manual of Standard Practice". Reinforcement bars shall not be tack welded, heated, or cut unless specifically detailed on the drawings or as approved by the Structural Engineer. The welding of reinforcing steel, metal inserts and connections in reinforced concrete shall conform to AWS D1.4 "Structural Welding Code - Reinforcing Steel" by AWS/WABO certified welders using E9018 or approved electrodes. Submit mill test report of material properties to determine carbon equivalent and minimum preheat temperatures. ASTM A706 reinforcement may be substituted for A615 reinforcement. Submit shop drawings of reinforcing steel for review by the Architect and Structural Engineer prior to fabrication. All wall reinforcement shop drawings shall be shown in elevation. STRUCTURAL AND MISCELLANEOUS STEEL C & L Shapes, plates and bars: ASTM A36, Fy = 36 KSI Wide Flange & WT Shapes: ASTM A992, Fy = 50 KSI Square & Rectangular HSS: ASTM A500, Grade C, Fy = 50 KSI Round HSS: ASTM A500, Grade C, Fy = 50 KSI Hillside washers: ASTM A48, Grade 30 hot dipped galvanized per ASTM F2329 Bolts: ASTM A307 machine bolts (MB). Anchor Rods (AR): Headed or threaded rod with heavy hex nut, ASTM F1554, Fy = 36 KSI with A563C (A563DH for galvanized) heavy hex nuts and F436-1 washers. Anchor Bolts (AB): Headed A307, Grade A or hooked ASTM A36. Threaded rod: ASTM A36, Fy = 36 KSI. Headed anchor stud (HAS): ASTM A108 Type A, Fy = 49 KSI for general use and Type B, Fy = 51 KSI for composite steel-concrete construction. Headed anchor studs shall be end welded per manufacturer's recommendations. Deformed bar anchors (DBA): ASTM A1064, Fy = 70 KSI, end welded per manufacturer's recommendations. Threaded Anchor Stud (TAS): ASTM A108, Fy = 49 KSI, end welded per manufacturer's recommendations. Welding electrodes or wires: AWS A5.1 or A5.5, E70XX; AWS A5.18, E70S-X; AWS A5.20, E7XT-X. Non-shrink Grout: "Masterflow 928" as manufactured by BASF Building Systems or approved equal with a minimum compressive strength of 5000 PSI. Erection and fabrication in accordance with AISC 360 "Specification for Structural Steel Buildings", AISC 341 "Seismic Provisions for Structural Steel Buildings", and AISC 303 "Code of Standard Practice for Steel Buildings and Bridges". Welding shall conform to AWS D1.1 "Standard Welding Code - Steel". All welding shall be performed by AWS/WABO Certified Welders whose certification meets "AWS Standards" in an approved fabricator shop or where Special Inspection is provided. All columns and beams to be from unsplined lengths unless noted otherwise on the drawings. All members to have "Standard" size holes unless noted as short-slotted (SSL) or long-slotted (LSL). Enlargement of holes by burning is not permitted. Masonry veneer lintel angles shall be hot-dipped galvanized with a minimum zinc coating of 1.5 ounces per square foot of surface area. Submit shop drawings showing sizes, dimensions and required connection details for review by the Architect and Structural Engineer prior to fabrication.																																								
	WOOD Sawn Lumber: Douglas Fir-Larch S-Dry 19% MC, WWSA 2017 Western Lumber grading rules furnished to the following minimum standards: <table><tr><th>Studs</th><th>2x4</th><th>Standard or better</th></tr><tr><td></td><td>2x6</td><td>#2 or better</td></tr><tr><td>Joist & Rafters</td><td>2x, 3x, 4x widths</td><td></td></tr><tr><td></td><td>6" to 14" deep</td><td>#2 or better</td></tr><tr><td>Top & Sill Plates</td><td>2x width</td><td>#2 or Construction Grade</td></tr><tr><td>Beams & Stringers</td><td>6x widths & larger</td><td>#1 or better</td></tr><tr><td>Posts and Timbers</td><td>6x widths & larger</td><td>#1 or better</td></tr><tr><td>T&G Decking</td><td>2x, 3x, 4x widths</td><td>Selected Grade</td></tr><tr><td>Treated Framing</td><td>2x, 3x, 4x widths</td><td>Hem-Fir #2 or better</td></tr></table> All wood bearing on concrete or masonry, if less than 4'-0" above grade, shall be preservative treated. Structural members shall not be cut for pipes, ducts, etc., unless specifically noted or detailed. Glulam members: All members shall conform to the latest edition of AITC 117 "Design Standard Specifications for Structural Glued Laminated Timber of Softwood Species". Simple span beams: Stress Class 24F-1.8E Combination Symbol 24F-V4 Continuous beams: Stress Class 24F-1.8E Combination Symbol 24F-V8 Cantilever beams: Stress Class 24F-1.8E Combination Symbol 24F-V8 Compression members: Combination Symbol 2 Tension members: Combination Symbol 3 Shop Drawings of glulam members to be submitted for review by the Architect and Structural Engineer prior to fabrication. Roof Sheathing: 5/8" (19/32" minimum) APA rated sheathing, Exposure 1, PRP-108, ESR-2586, Span Rating 40/20. Provide tongue & groove edges, ply clips, or blocking at all panel edges where roof slopes are less than 2:12, or spacing of support framing exceeds 32" oc, or where noted on the drawings. Lay sheathing with face grain perpendicular to supports continuous over 2 or more spans with end joints staggered. Allow 1/8" spacing at panel ends and edges. Attach to framing with 8d @ 6" oc at panel edges and 12" oc at intermediate supports unless noted otherwise. Wall Sheathing: 1/2" (15/32" minimum) APA rated sheathing, Exposure 1, PRP-108, ESR-2586, Span Rating of 24/0. Provide solid blocking at all panel edges. Lay sheathing with face grain perpendicular to supports continuous over 2 or more spans. Allow 1/8" spacing at panel ends and edges. Attach to framing with 8d @ 6" oc at panel edges and 12" oc at intermediate supports unless noted otherwise. Framing anchors and connectors: "Simpson" or approved equal as indicated on drawings. Install per manufacturer's recommendations. Bolt heads and nuts in contact with wood shall have washers. For nailing not shown on drawings, use IBC Fastening Schedule, Table 2304.10.1. Nails shall not be overdriven. Preservative-Treated Wood: All preservative-treated wood (PT) and fire-treated wood (FT) shall be treated in compliance with the American Wood Protection Association (AWPA) standard U1-09. The AWPA Use Category System (UCS) shall be used to determine an appropriate AWPA-approved preservative system and retention level for specific commodities and end-uses. Unless otherwise specified, species and retention levels used in the preservative system shall be compliant with those listed in Section 6 of AWPA-U1-09. Treatments CCA-C, ACQ-A, and ACQ-B are prohibited. All framing hardware, connectors, etc. in contact with treated wood shall be galvanized per ASTM A653 or stainless steel. All framing hardware fasteners into treated wood shall be galvanized per ASTM A153 or stainless steel.										Studs	2x4	Standard or better		2x6	#2 or better	Joist & Rafters	2x, 3x, 4x widths			6" to 14" deep	#2 or better	Top & Sill Plates	2x width	#2 or Construction Grade	Beams & Stringers	6x widths & larger	#1 or better	Posts and Timbers	6x widths & larger	#1 or better	T&G Decking	2x, 3x, 4x widths	Selected Grade	Treated Framing	2x, 3x, 4x widths	Hem-Fir #2 or better				
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	POST-INSTALLED ANCHORS All post-installed anchors shall have current ICC ESR Reports indicating code-approved installation requirements. Contractor shall comply with all requirements of the ESR report. Substitution of post-installed anchors for anchor bolts, anchor rods, or rebar dowels is not allowed without written approval of the Structural Engineer. Special Inspection is required for all post-installed anchors. Special Inspection for Contractor-requested substitutions (if approved) shall be provided at no additional cost to the Owner. Adhesive Anchors (AA) in Concrete: ASTM A307, Grade C threaded rod with Simpson SET-XP adhesive or Hilti "HAS-E" threaded rod or Grade C threaded rod with HIT-HY 200 adhesive or approved equal. Concrete to have aged a minimum of 21 days prior to installation. Adhesive Dowels (AD) in Concrete: ASTM A615 deformed bar with Simpson SET-XP adhesive or Hilti HIT-HY 200 adhesive or approved equal. Concrete is to have aged a minimum of 21 days prior to anchor installation. Adhesive Anchors (AA) in Solid-Grouted Concrete Masonry: ASTM F1554, Grade 36 threaded rod with Simpson SET-XP adhesive or Hilti "HIS" threaded rod or Grade C threaded rod with HIT-HY 270 adhesive or approved equal. Adhesive Anchors (AA) in Hollow Concrete Masonry: ASTM F1554, Grade 36 threaded rod with Simpson SET-XP adhesive in screen tube or Hilti "HIT-IC" threaded rod with HIT-HY 270 adhesive inHIT-SC screen tube or approved equal. Expansion Bolts (EB) in Concrete: Simpson Strong Bolt 2 or Hilti Kwik Bolt TZ or approved equal. Expansion Bolts (EB) in Solid-Grouted Concrete Masonry: Simpson Wedge-All or Hilti Kwik Bolt 3 or approved equal. Screw-In Anchors (SA) in Concrete: Simpson Titen HD or Hilti KWIK HUS-EZ or approved equal. Powder Actuated Fasteners (PAF) into Concrete: Hilti X-U, (.157" dia) fasteners with a minimum embedment of 3/4" (1" length) into concrete of a minimum 3" thickness. Do not install PAF fasteners until concrete has attained specified 28 day design strength. Powder Actuated Fasteners (PAF) into Steel: Hilti X-U 15 (.145" dia) fasteners into steel of a minimum 3/8" thickness.																																								
	QUALITY ASSURANCE and SPECIAL INSPECTIONS A qualified special inspector(s) shall be engaged by the Owner or Owner's representative to conduct continuous and periodic inspections as tabulated on sheet S1.02. Construction activity shall be coordinated with required inspections and testing requirements by the Contractor. Construction deficiencies identified by the special inspection process shall be remedied by the Contractor at no cost to the Owner.																																								

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DRAWN BY CJC
CHECKED BY LAH
JOB NUMBER 22103
REVISIONS

LSB
CONSULTING
ENGINEERS
523 EAST THIRD AVENUE
SPOKANE, WA 99202
P 509.323.9292
F 509.747.7115

LAURENCE A. HALL
STATE OF WASHINGTON
REGISTERED
PROFESSIONAL ENGINEER
12/20/23

WSU GOLF TEAM
CLUBHOUSE UPGRADE
EXTERIOR & BLDG ADDITION
WASHINGTON STATE UNIVERSITY
PULLMAN, WA 99163

DATE
01/26/2024

SHEET NAME
GENERAL
STRUCT
NOTES

SHEET
S1.01

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C

B

A

SPECIAL INSPECTIONS & TESTING

GENERAL

The Owner or the Owner's agent shall employ an approved agency to perform inspections and testing during construction in accordance with IBC sections 1704, 1705 and 1707 and the Statement of Special Inspections below. These inspections are in addition to the inspections required per IBC section 110.

Special Inspectors shall be qualified persons with experience in the types of construction being inspected. Special Inspectors shall provide written documentation of WABO or equivalent certification acceptable to the Agency Having Jurisdiction.

Off-site work and fabrications do not require special inspection if the fabricator is registered and approved to perform such work by the Agency Having Jurisdiction. Approval is based on the fabricator's written procedures and quality control programs.

CONTRACTOR RESPONSIBILITIES

The Contractor shall provide reasonable access to all work requiring special inspection and notify the Inspection Company in sufficient time to schedule such inspections. A copy of all Special Inspection reports shall be maintained at the job site and made available upon request to the Authority Having Jurisdiction and the project Engineer of Record.

STATEMENT OF SPECIAL INSPECTION

Special inspection to be provided per IBC 1705 for materials, systems, components and work performed are as follows:

Freq = Frequency

Req = Required

C = Continuous Special Inspection

P = Periodic Special Inspection

SOILS AND FOUNDATION

Provide Special Inspection for the following to determine compliance with the Geotechnical Report and the Construction Documents:

SOILS AND FOUNDATION

System or Material	Code Reference	Freq	Req
1. Verify Footing Bearing Capacity	IBC 1705.6	P	X
2. Verify Excavations are to proper depth and have reached proper material	IBC 1705.6	P	X
3. Classification and Testing of Compacted Fill Materials	IBC 1705.6	P	X
4. Prior to the placement of Compacted Fill, observe subgrade and verify the site has been prepared properly	IBC 1705.6	P	X
5. Verify use of proper materials, densities and lift thicknesses during placement and compaction of fill	IBC 1705.6	C	X

CONCRETE

System or Material	Code Reference	Freq	Req
1. Inspect reinforcing steel and verify placement	ACI 318 Chap 20, 25.2, 25.3, 26.6.1-26.6.3, IBC 1908.4	P	X
2. Cast-in-Place anchors in concrete	ACI 318 17.8.2	P	X
3. Verify use of required mix designs	ACI 318 19, 26.4.3 & 26.4.4 IBC 1904.1, 1904.2, 1908.2, 1908.3	P	X
4. Sampling of fresh concrete for strength tests, slump and air content tests and to determine temperature of the concrete	ACI 318 26.5 & 26.12 ASTM C172 & C31, IBC 1908.10	C	X
5. Inspect concrete and shotcrete placement for proper application techniques	ACI 318 26.5 & 26.13.3.2(a) IBC 1908.6, 1908.7 & 1908.8	C	X
6. Verify maintenance of specified curing temperature and techniques	ACI 318 26.5.3 -26.5.5 IBC 1908.9	P	X
7. Inspect prestressed concrete for application of prestressing forces & grouting of bonded tendons	ACI 318 26.10 & 26.13.3.2	C	
8. Inspect erection of precast concrete members	ACI 318 26.9 & 26.13.3.3	P	
9. Verification of in-situ concrete strength prior to stressing tendons in post-tensioned concrete	ACI 318 26.11.2	P	
10. Verification of in-situ concrete strength prior to removal of forms and shores from elevated beams and slabs	ACI 318 26.11.2	P	
11. Inspect formwork for shape, location and dimensions of the member being formed	ACI 318 26.11.1.2(b)	P	
12. Placement of Reinforcing steel & Concrete in Special Moment Frames	ACI 318 18.2.5, 18.2.6, 26.13	C	
13. Placement of Reinforcing steel & Concrete in Special Shear Walls	ACI 318 18.2.5, 18.2.6, 26.13	C	
14. Concrete Placement for composite slabs	AISC 341 J9	C	
15. Stressing of Tendons in Post-tensioned concrete	ACI 318 26.10.2 & 26.13.3.2	C	
16. Post-Installed Anchors in Hardened Concrete			
a. Adhesive anchors installed horizontally or upwardly inclined and resisting sustained tension loads	ACI 318 17.8.2.4, ICC Report	C	X
b. Mechanical anchors and other adhesive anchors	ACI 318 17.8.2, ICC Report	P	X

WOOD

System or Material	Code Reference	Freq	Req
1. Fabrication of prefabricated structural elements	IBC 1704.2.5	P	X
2. Prefabricated Wood Shear Walls	IBC 2306.3 & ICC Report	P	X
3. Fabrication of high-load diaphragms: - Verify panel grade and thickness. - Verify nominal size of framing members at adjoining panel joints. - Verify nail or staple diameter and length, number of fastener lines and spacing between fasteners at each line and at edge margins.	IBC 1705.5.1 & 2306.2	P	
4. Verification of the installation of temporary and permanent restraint bracing for individual trusses spanning 60' or greater	IBC 1705.5.2	P	

MASONRY - LEVEL 2

System or Material	Code Reference	Freq	Req
1. Compliance with approved submittals	TMS 602-16 Section 1.5	P	X
2. Verify f'm prior to construction	TMS 602-16 Section 1.4B	P	X
3. Observe preparation of grout and mortar specimens and/or prism	TMS 602-16 Section 1.4B.2.a.3, 1.4B.2.b.3, 1.4B.2.c.3, 1.4B.3, 1.4B.4	P	X
4. Verify as Masonry Construction Begins			
a. Sample panel construction	TMS 602-16 Section 1.6D	P	X
b. Proportions of site-prepared mortar	TMS 602-16 Section 2.1, 2.6A	P	X
c. Construction of mortar joints	TMS 602-16 Section 3.3B	P	X
5. Verify Prior to Grouting			
a. Grout space	TMS 602-16 Section 3.2D & 3.2F	P	X
b. Grade, type and size of reinforcement, connectors and anchor bolts	TMS 602-16 Section 3.4 & 3.6A	P	X
c. Placement of reinforcement, connectors and anchor bolts	TMS 602-16 Section 3.2E & 3.4 TMS 402-16 Section 6.1, 6.3.1, 6.3.6 & 6.3.7	P	X
d. Proportions of site-prepared grout	TMS 602-16 Section 2.6B	P	X
e. Construction of mortar joints	TMS 602-16 Section 3.3B	P	X
6. Verify During Construction			
a. Slump Flow and Visual Stability Index	TMS 602-16 Section 1.5 & 1.6.3	P	X
b. Size and location of structural elements	TMS 602-16 Section 3.3F	P	X
c. Type, size and location of anchors and details of anchorage of masonry to structural members, frames or other construction	TMS 402-16 Section 1.2.1(e), 6.2.1 & 6.3.1	P	X
d. Preparation, construction, and protection of masonry during cold weather (below 40 degrees) or hot weather (above 90 degrees)	TMS 602-16 Section 1.8C & 1.8D	P	X
e. Placement of grout	TMS 602-16 Section 3.5	C	X

SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE

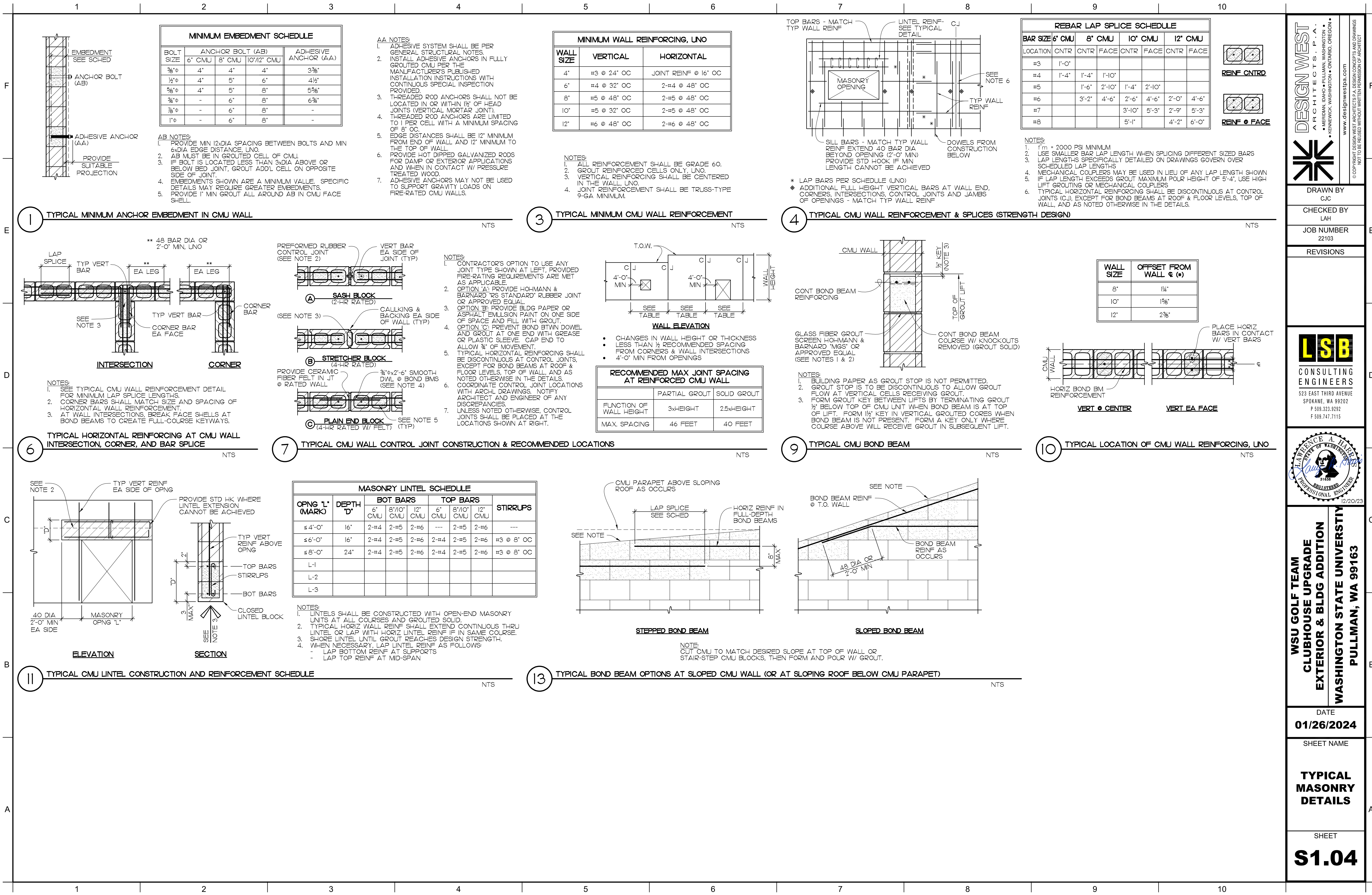
System or Material	Code Reference	Freq	Req
1. Structural Wood			
a. Field gluing of diaphragm and shear wall elements of the seismic force resisting system	IBC 1705.12.2	C	
b. Nailing, bolting, anchoring and other fastening of elements of the seismic force resisting system including shear walls, diaphragms and chords, collectors, braces, shear wall anchorage and holdowns (not required when fastener spacing of sheathing is > 4")	IBC 1705.12.2	P	X

ABBREVIATIONS

AA	ADHESIVE ANCHOR	K	KIP
AB	ANCHOR BOLT	KSF	KIPS PER SQUARE FOOT
AD	ADHESIVE DOWEL		
ADDL	ADDITIONAL	L OR L	ANGLE
AFF	ABOVE FINISH FLOOR	LBS	POUNDS
ALT	ALTERNATE	LL	LIVE LOAD
&	AND	LLBB	LONG LEGS BACK TO BACK
APPROX	APPROXIMATE	LLH	LONG LEG HORIZONTAL
AR	ANCHOR ROD	LLV	LONG LEG VERTICAL
ARCH	ARCHITECT	LNTL	UNITE
ARCHL	ARCHITECTURAL	LONG	LONGITUDINAL
@	AT	LSL	LONG SLOTTED
		LVL	LAMINATED VENEER LUMBER
B.O.	BOTTOM OF	MAS	MASONRY
BOT	BOTTOM	MAX	MAXIMUM
BLDG	BUILDING	MB	MACHINE BOLT
BLKG	BLOCKING	MECH	MECHANICAL
BM	BEAM	MEZZ	MEZZANINE
BO	BOTTOM OF	MFR	MANUFACTURER
BRG	BEARING	MIN	MINIMUM
BSMT	BASEMENT	MISC	MISCELLANEOUS
BTWN	BETWEEN	ML	MICROLLAM
		MO	MASONRY OPENING
		MTL	METAL
C	CHANNEL		
CB	CARRIAGE BOLT	# OR NO.	NUMBER
CDF	CONTROLLED DENSITY FILL	NC	NOT IN CONTRACT
CL OR E	CENTERLINE	NOM	NOMINAL
CLG	CEILING	NS	NEAR SIDE
C-C	CENTER TO CENTER	NTS	NOT TO SCALE
CF	CUBIC FEET		
CIP	CAST-IN-PLACE	OC	ON CENTER
CJ	CONTROL JOINT	OD	OUTSIDE DIAMETER
CLR	CLEAR	OF	OUTSIDE FACE
CMU	CONCRETE MASONRY UNIT	OH	OVERHEAD
CONTRD	CENTERED	O-O	OUT TO OUT
COL	COLUMN	OPNG	OPENING
CONC	CONCRETE	OPP	OPPOSITE
CONN	CONNECTION		
CONSTR	CONSTRUCTION	PAF	POWDER ACTUATED FASTENER
CONT	CONTINUOUS	PC	PRECAST
CONTR	CONTRACTOR	PL OR R	PLATE
CTSK	COUNTERSINK	PLF	POUNDS PER LINEAL FT
CTR	CENTER	PCF	POUNDS PER CUBIC FT
CY	CUBIC YARD	PLYWD	PLYWOOD
		PLCS	PLACES
DBA	DEFORMED BAR ANCHOR	PSF	POUNDS PER SQ FT
DBL	DOUBLE	PSI	POUNDS PER SQ IN
DEG	DEGREE	PSL	PARALLAM
DIAG	DIAGONAL	PT	PRESSURE TREATED
DIA	DIAMETER		
DIM	DIMENSION	RAD	RADIUS
DL	DEAD LOAD	R	RISK
DOWN	DOWN	REIN	REINFORCEMENT
DP	DEEP	REF	REFERENCE
DTL	DETAIL	REQD	REQUIRED
DWG	DRAWING	REQMTS	REQUIREMENTS
DWL	DOWEL	REV	REVISION
		RO	ROUGH OPENING
(E)	EXISTING	RM	ROOM
EA	EACH		
EXP	EXPANSION BOLT	SA	SCREWHN ANCHOR
FACE	EACH FACE	SCHED	SCHEDULE
EJ	EXPANSION JOINT	SF	SQUARE FEET
ELEV	ELEVATION	SHT	SHEET
EMBED	EMBEDMENT	SHTG	SHEATHING
EN	EDGE NAIL	SIM	SIMILAR
ENGGR	ENGINEER	SPCS	SPACES
EQ	EQUAL	SPECS	SPECIFICATIONS
EW	EACH WAY	SQ	SQUARE
EXP	EXPANSION	SSL	SHORT SLOTTED
EXIST	EXISTING	SJ	SLAB JOINT
EXTR	EXTERIOR	SS	STAINLESS STEEL
		STD	STANDARD
FD	FLOOR DRAIN	STIFF	STIFFENER
FDN	FOUNDATION	STGRD	STAGGERED
FIN	FINISH	STL	STEEL
FLR	FLOOR	STRUC	STRUCTURAL
FN	FIELD NAIL	SYM	SYMMETRICAL
FO	FACE OF		
FOC	FACE OF CONCRETE	TAS	THREADED ANCHOR STUD
FRG	FRAMING	T & B	TOP & BOTTOM
FS	FAR SIDE	THK	THICK
FT	FEET	THRD	THREADED
FTG	FOOTING	TN	TOE NAIL
		TOP	TOP OF
GAGE	GAGE	TRANS	TRANSVERSE
GALV	GALVANIZE	TS	TUBE STEEL
GL	GLUED LAMINATED	TYP	TYPICAL
GR	GRADE		
GWB	GYPSUM WALLBOARD	UA	UNDERCUT ANCHOR
		UNO	UNLESS NOTED OTHERWISE
H	HORIZONTAL		
HAS	HEADED ANCHOR STUD	V	VERTICAL
HDR	HEADER	(V)	VERIFY
HORIZ	HORIZONTAL	VERT	VERTICAL
HSS	HIGH STRENGTH BOLT		
HSS	HOLLOW STRUCTURAL SECTION	W/	WITH
HT	HEIGHT	W/O	WITHOUT
HWH	HEX WASHER HEAD	WD	WOOD
		WF	WIDE FLANGE
ID	INSIDE DIAMETER	WP	WORK POINT
I.F.	INSIDE FACE	WS	WATERSTOP
IN	INCHES	WT	WEIGHT
INTR	INTERIOR	WWF	WELDED WIRE FABRIC
		x	TIMES OR BY
JST	JOIST		
JT	JOINT		
JB	JOIST BEARING		

LEGEND

	JOIST BEARING ELEVATION
	TOP OF WALL ELEVATION
	TOP OF PARAPET ELEVATION
	TOP OF BEAM ELEVATION
	TOP OF SLAB ELEVATION
	TOP OF CONCRETE ELEVATION
	TOP OF FOOTING ELEVATION
	STEPPED FOOTING
	ELEVATION CHANGE
	CMU WALL
	WOOD STUD WALL
	METAL STUD WALL
	FOOTING MARK
	SHEAR WALL MARK
	REVISION MARK
	HOLDOWN MARK
	SLAB JOINT
	CONTROL JOINT
	FINISH FLOOR ELEVATION
	BM SIZE
	SHEAR STUDS SEE I7&18/SO03
	BEAM DESIGNATION
	CAMBER
	TOP OF BM ELEVATION
	ROOF TOP MOUNTED MECH UNIT WEIGHT AS NOTED
	MECH UNIT MOUNTED UNDER AND FROM FRAMING W/WEIGHT
	ELEVATION LETTER
	ELEVATION
	SHEET NUMBER
	SECTION NUMBER
	BUILDING SECTION
	SHEET NUMBER
	DETAIL NUMBER
	DETAIL
	SHEET NUMBER
	THREADED ANCHOR STUD
	TOP & BOTTOM
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	THREADED
	TOE NAIL



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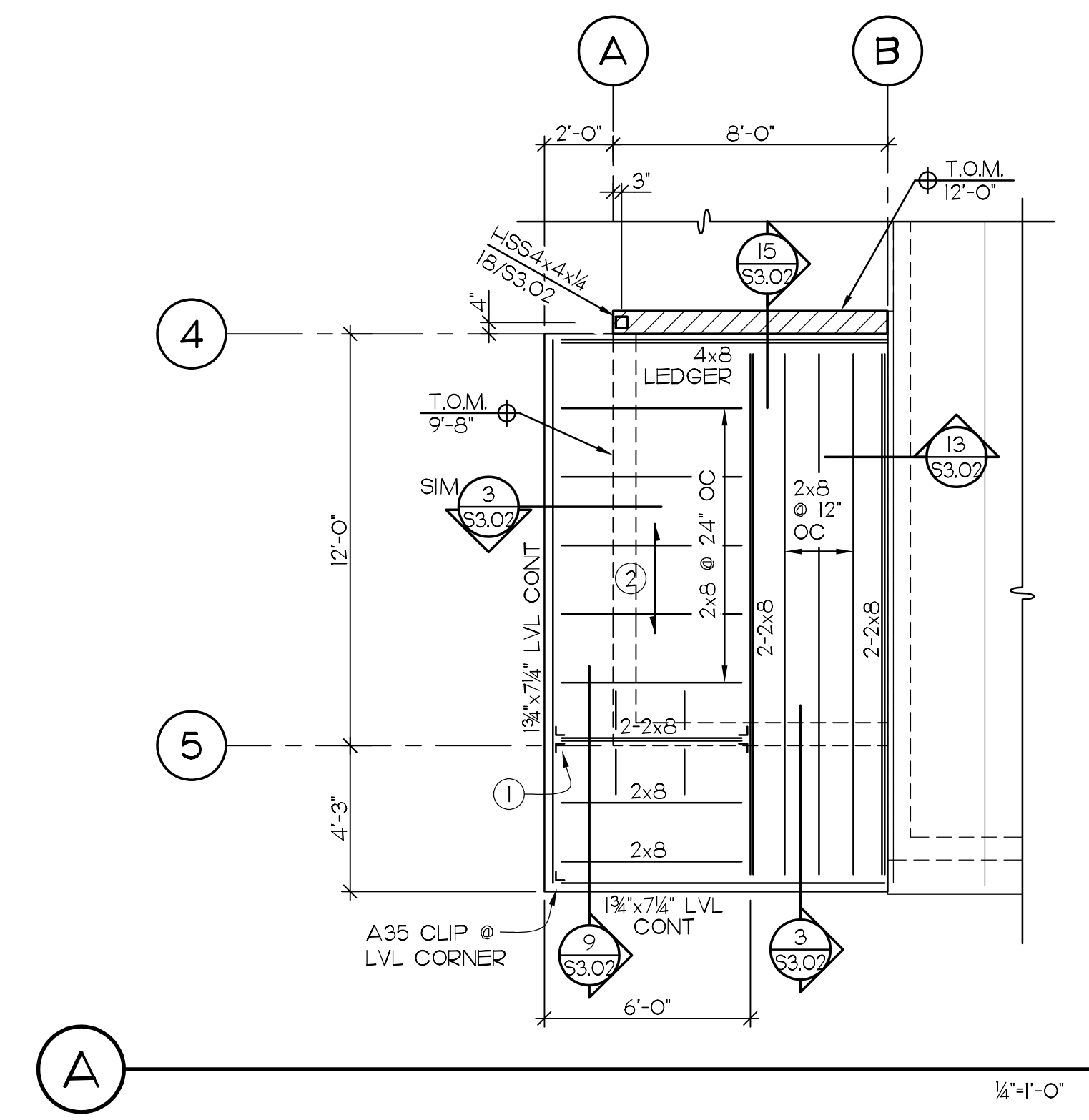
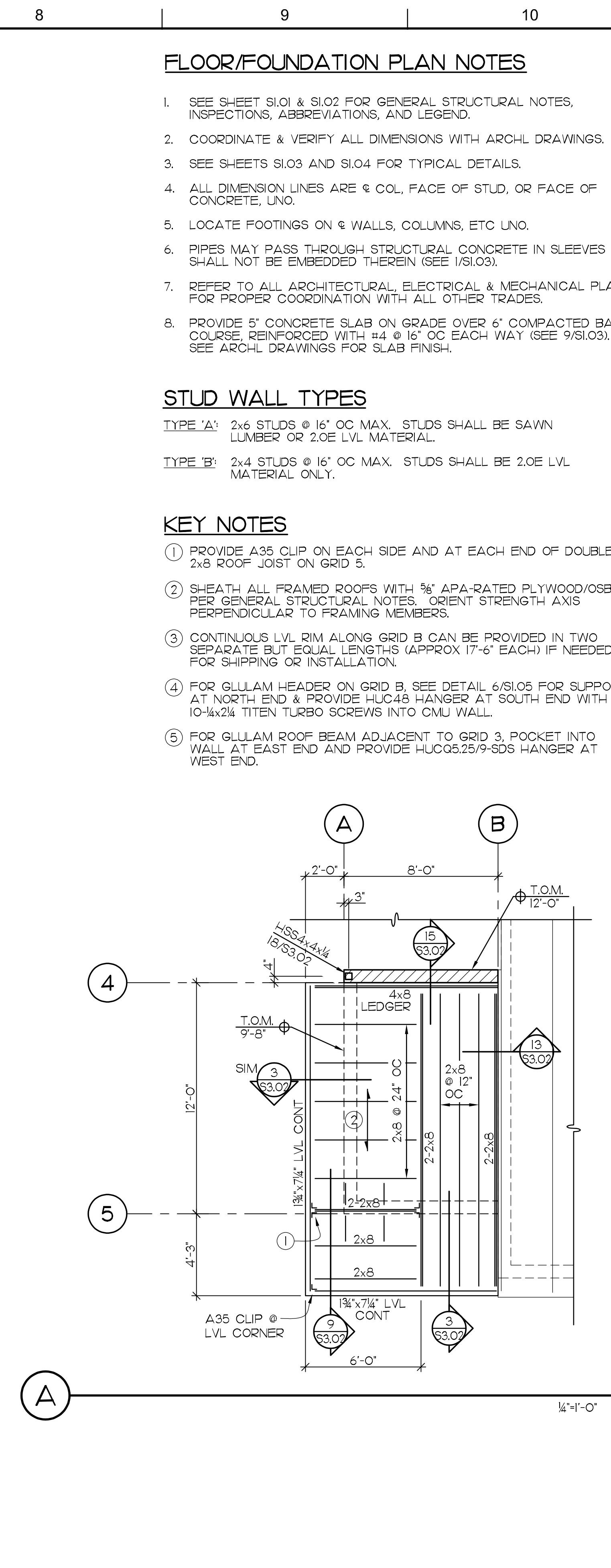
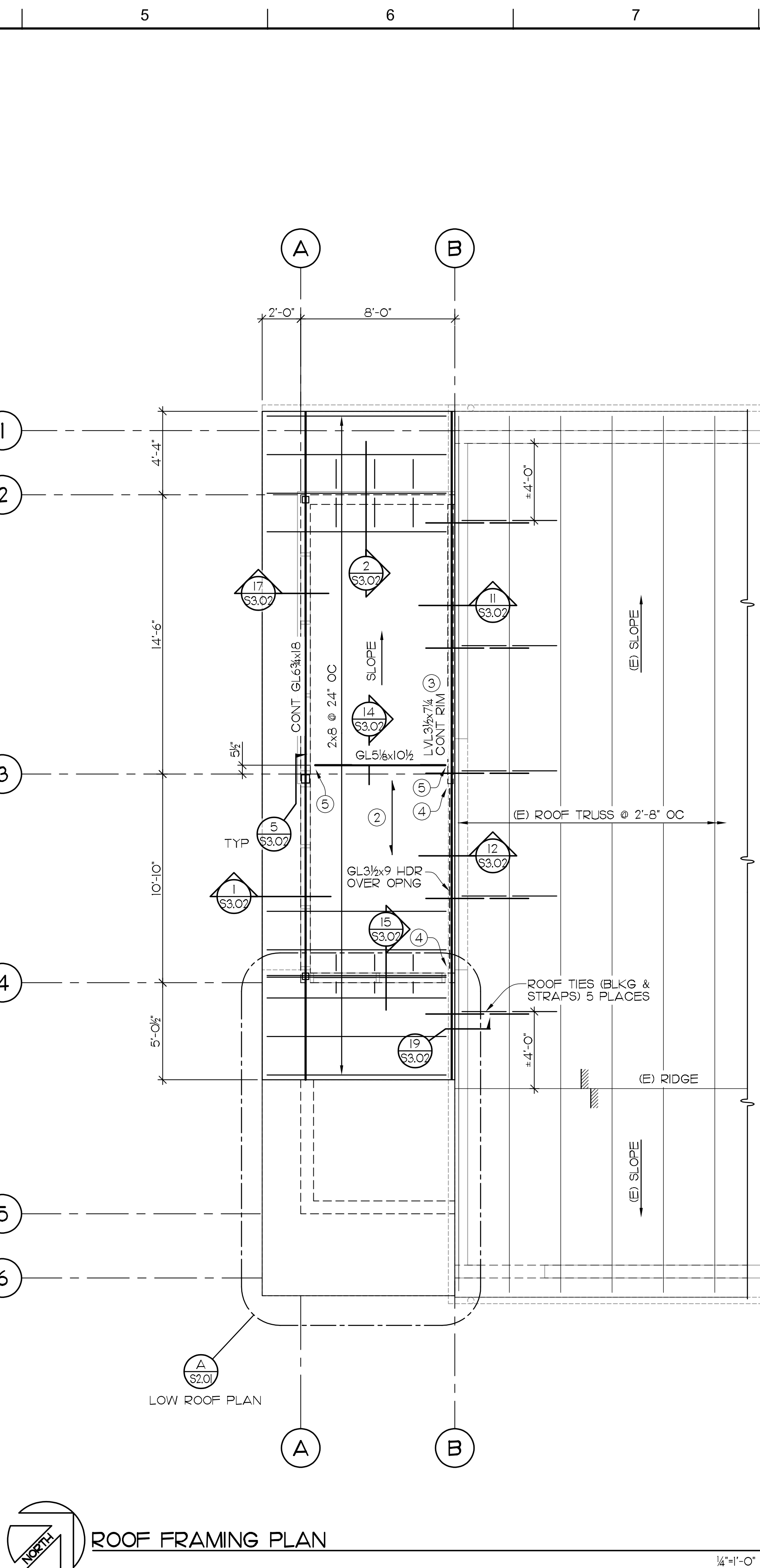
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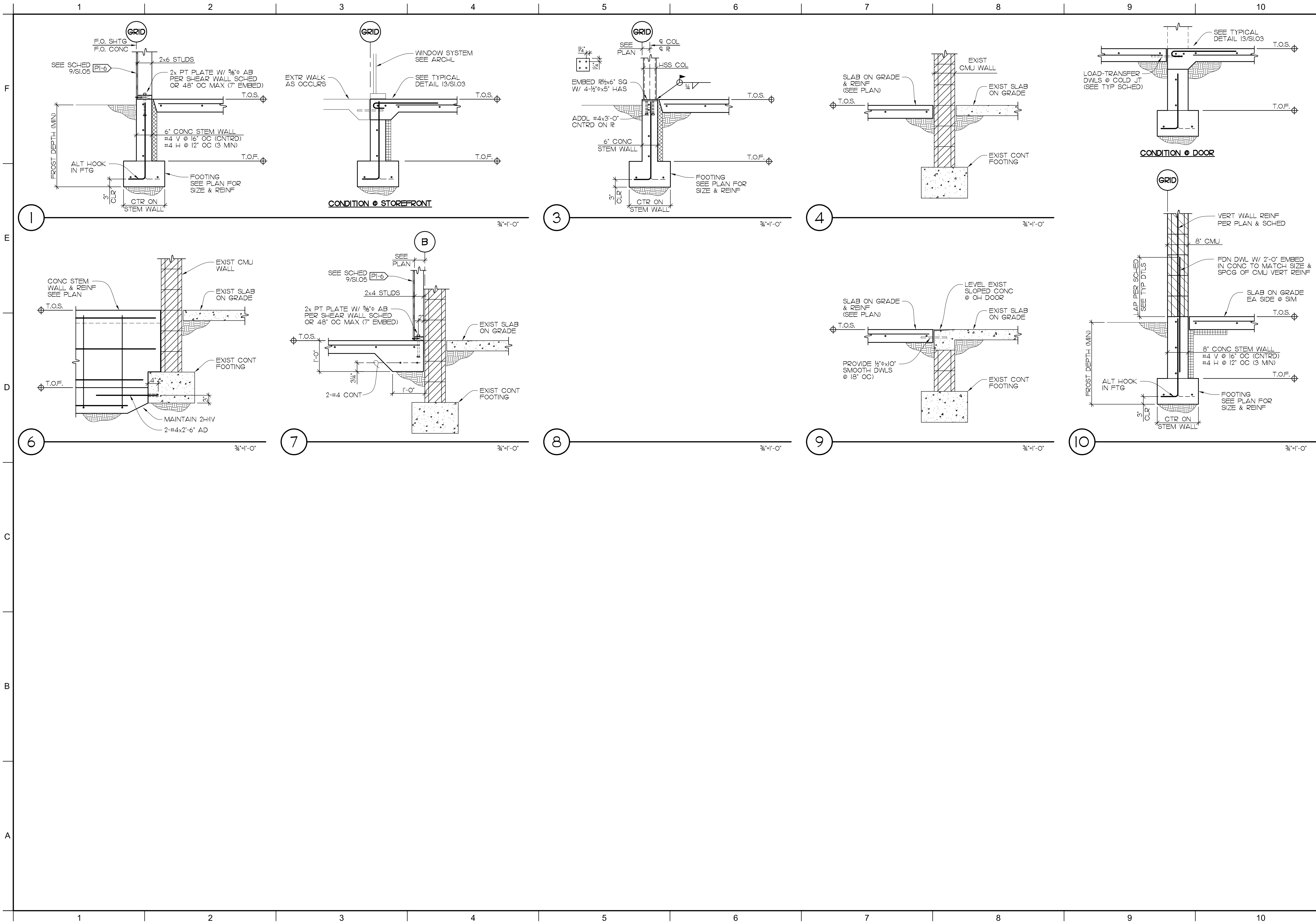
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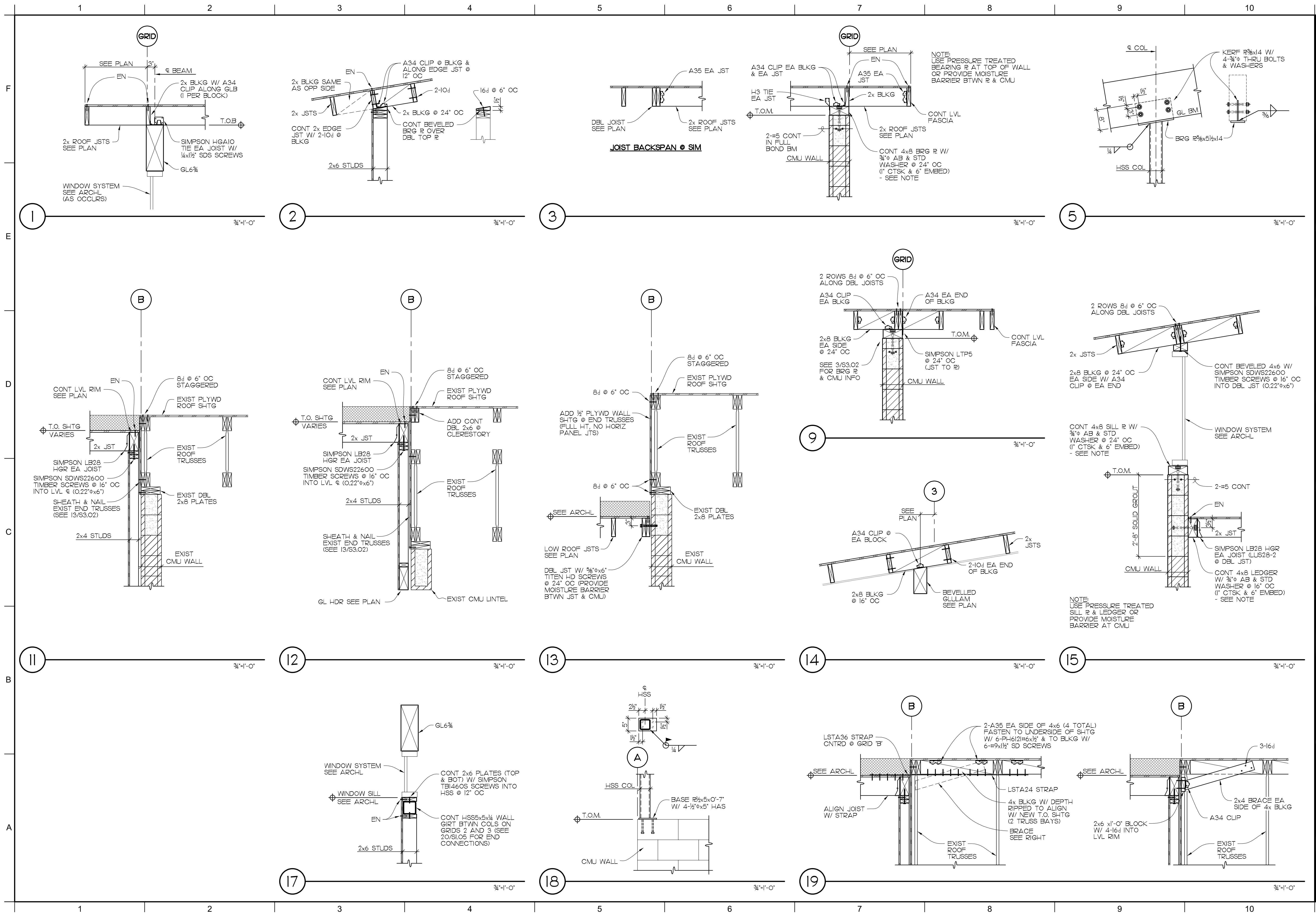
WSU GOLF TEAM
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EXTERIOR & BLDG ADDITION
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F 509.747.7115

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REGISTERED
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EXHAUST FAN SCHEDULE													
#	MFR	MODEL	TYPE	SERVICE	CFM	ESP (")	DRIVE	FAN RPM	ELECTRICAL			WEIGHT (LBS)	NOTES
									V/PH	MCA	MCA		
EF-1	PANASONIC	WHISPER GREEN SELECT FV-1115VKZ	CEILING MOUNT	BATHROOM EXHAUST	150	0.1	DIRECT ECM	700	120/1	0.53	15	15	①
NOTES: ① PROVIDE WITH THE FOLLOWING: - CLASS 1 MOTORIZED EXHAUST DAMPER TO OPEN WHEN FAN RUNS - FACTORY DISCONNECT - EC MOTOR WITH MANUAL SPEED ADJUSTMENT DIAL ON MOTOR (CAPABLE OF CONTROL BY OTHERS)													

AIR OUTLETS SCHEDULE												
#	MFR	MODEL	SERVICE	TYPE	MATERIAL	MOUNTING	BORDER	PATTERN	FINISH	OTHER	NOTES	
①	PRICE	535	RETURN/ TRANSFER/ EXHAUST	GRILLE	STEEL	CEILING/ SURFACE	FOR APPLICATION	45° FIXED	WHITE	1/2" BLADE SPACING	①②③	
NOTES: ① PAINT THE INSIDE SURFACE OF THE DUCTWORK CONNECTION FLAT BLACK, UNLESS OTHERWISE PROVIDED WITH DUCT LINER. ② PROVIDE BALANCING DAMPER IN BRANCH DUCT SERVING AIR OUTLET AT TAKEOFF FROM TRUNK DUCT. ③ PROVIDE TRANSITION FROM AIR OUTLET NECK TO BRANCH DUCT AS REQUIRED.												

ELECTRIC HEATER SCHEDULE							
#	MFR	MODEL	MOUNTING	COLOR	ELECTRICAL		NOTES
					WATTS	V/PH	
EH-1	BERKO	FRC	RECESSED WALL 12" AFF	NORTHERN WHITE	3000	208/1	①
EH-2	BERKO	GFR	RECESSED WALL 12" AFF	NORTHERN WHITE	1500	208/1	①
EH-3	BERKO	HUHTA	SUSPENDED 8' AFF	NORTHERN WHITE	2000	208/1	①
EH-4	BERKO	SRA	RECESSED WALL 12" AFF	NORTHERN WHITE	1500	208/1	
NOTES: ① PROVIDE WITH BUILT-IN TAMPER-RESISTANT T-STAT AND CONCEALED POWER ON/OFF SWITCH FOR MAINTENANCE							

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ENGINEERS

www.msi-engineers.com

ARON S. DONNELLY
STATE OF WASHINGTON
43373
REGISTERED
PROFESSIONAL ENGINEER

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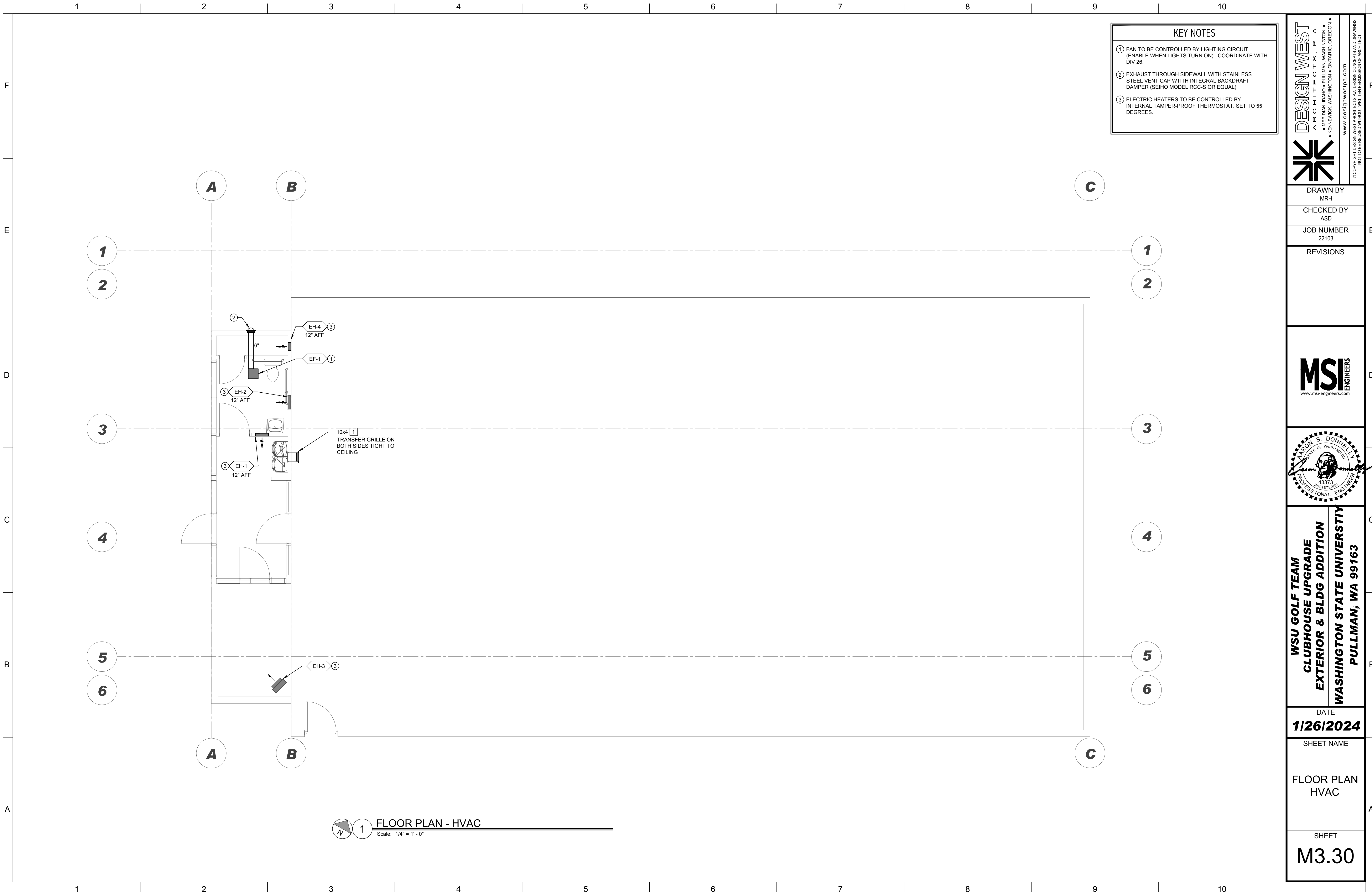
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DATE
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MECHANICAL
EQUIPMENT
SCHEDULES

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KEY NOTES

1

FAN TO BE CONTROLLED BY LIGHTING CIRCUIT (ENABLE WHEN LIGHTS TURN ON). COORDINATE WITH DIV 26.

2

EXHAUST THROUGH SIDEWALL WITH STAINLESS STEEL VENT CAP WITH INTEGRAL BACKDRAFT DAMPER (SEIHO MODEL RCC-S OR EQUAL)

3

ELECTRIC HEATERS TO BE CONTROLLED BY INTERNAL TAMPER-PROOF THERMOSTAT. SET TO 55 DEGREES.

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STATE OF WASHINGTON

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DATE

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SHEET NAME

FLOOR PLAN HVAC

SHEET

M3.30

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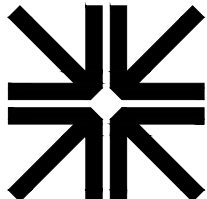
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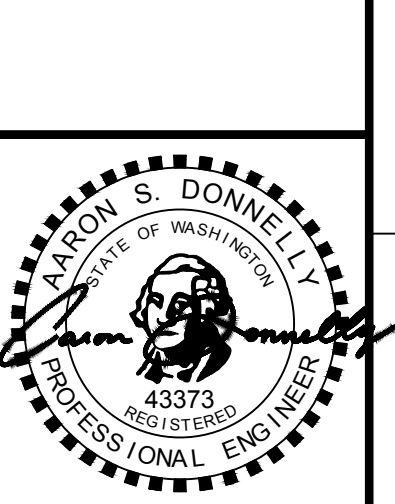
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PLUMBING FIXTURES																		
#	FIXTURE	MFR	MODEL #	MOUNTING	MATERIAL	SIZE	MFR & MODEL# OF FAUCET & VALVE	DRAIN	TRAP	CARRIER	ACCESSORIES	W	V	HW	CW	G	STOPS	NOTES
P1	ADA WATER CLOSET	AMERICAN STANDARD	MADERA 3043001.060	FLOOR	VITREOUS CHINA	ELONGATED BOWL	SLOAN 111-5M0-DFB-TP	INTEGRAL	INTEGRAL	-	SEAT BEMIS	4"	2"	-	1"	-	INTEGRAL	①②
P2	ADA LAVATORY	AMERICAN STANDARD	LUCERNE 0355.012	WALL	VITREOUS CHINA	SINGLE BOWL 20"x18"	" CHICAGO " 116.592.AB.1	ELKAY LKAB174	1 1/4"x17GA C.P.	JAY R. SMITH 0700	-							①②③
P3	WATER COOLER / BOTTLE FILLER	ELKAY	LVRGCRNTL8WSK	WALL	STAINLESS STEEL	DUAL HEIGHT	INTEGRAL	INTEGRAL	1 1/4"x17GA C.P.	JAY R. SMITH 0834 SERIES	-	1-1/2"	1-1/2"	-	1/2"	-	BRASS CRAFT TO SUIT	①② 120V/60HZ
NOTES: ① MOUNT PER ADA REQUIREMENTS. ② CAULK FIXTURE AT FLOOR, COUNTERTOP AND/OR WALL SURFACE WITH CAULKING. ③ PROVIDE WITH PLUMBEREX TRAP & SUPPLIES WRAP KIT. ⑥ PROVIDE WITH OPTIONS KMH AND KH36 ⑦ CONSULT ARCHITECT FOR COLOR. ⑧ SINK TO BE CUSTOM FABRICATED WITH THE DIMENSIONS INDICATED AND SINK LEDGE ON EITHER SIDE FOR MOUNTING OF FAUCET.																		

WALL HYDRANT SCHEDULE				
#	MANUFACTURER	MODEL #	SIZE	NOTES
HB-1	WOODFORD	MODEL 24	3/4"	②
FPWH-1	JAY R. SMITH	MODEL 5519	3/4"	AUTO DRAINING, VACUUM BREAKER, LOOSE - TEE KEY ①
NOTES: ① VERIFY WALL THICKNESS. ② LOCATED IN MECHANICAL ROOM				

BACKFLOW PREVENTION DEVICES						
#	MANUFACTURER	MODEL #	SIZE	SERVICE	ACCESSORIES	NOTES
BFP-1	WATTS	#LF009 SERIES	2"	DOMESTIC WATER	-	①
BFP-2	WATTS	#LF009 SERIES	2"	DOMESTIC WATER	-	①
NOTES: ① PROVIDE WITH AIR GAP FITTING AND DRAIN UNDIMINISHED IN SIZE TO FLOOR DRAIN.						

ELECTRIC TANKLESS WATER HEATER SCHEDULE											
#	MFR	MODEL	GALLONS	KW INPUT	FLOW (GPM)	OUTLET TEMP (°F)	TEMP RISE (°F)	ELECTRICAL			NOTES
								V/PH	MCA	MOCP	
WH-1	CHRONOMITE	CM-30L/208	0	6.24	0.5	110	85	208/1	30	40	①
NOTES: ① INCLUDE WALL SLEEVE TO CONCEAL HEATER AND PRE-WIRED DISCONNECT SWITCH											

FLOOR DRAIN SCHEDULE									
#	MFR	MODEL	TYPE	BODY		STRAINER/GRATE		VARIATION	NOTES
				STYLE	MATERIAL	STYLE	MATERIAL		
FD-1	JAY R. SMITH	2005-A-P-U-Y	FLOOR DRAIN	-	CAST IRON	VANDEL-PROOF ROUND	NICKEL BRONZED	TRAP PRIMER	①
FS-1	JAY R. SMITH	3120Y-12	FLOOR SINK	-	CAST IRON	-	NICKEL BRONZED	TRAP PRIMER	①
NOTE: ① PROVIDE WITH SUFFIX -C WHEN LOCATED IN MECHANICAL ROOMS AND SIMILAR SPACES.									

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AKARON S. DONNELLY

STATE OF WASHINGTON

43373

REGISTERED PROFESSIONAL ENGINEER

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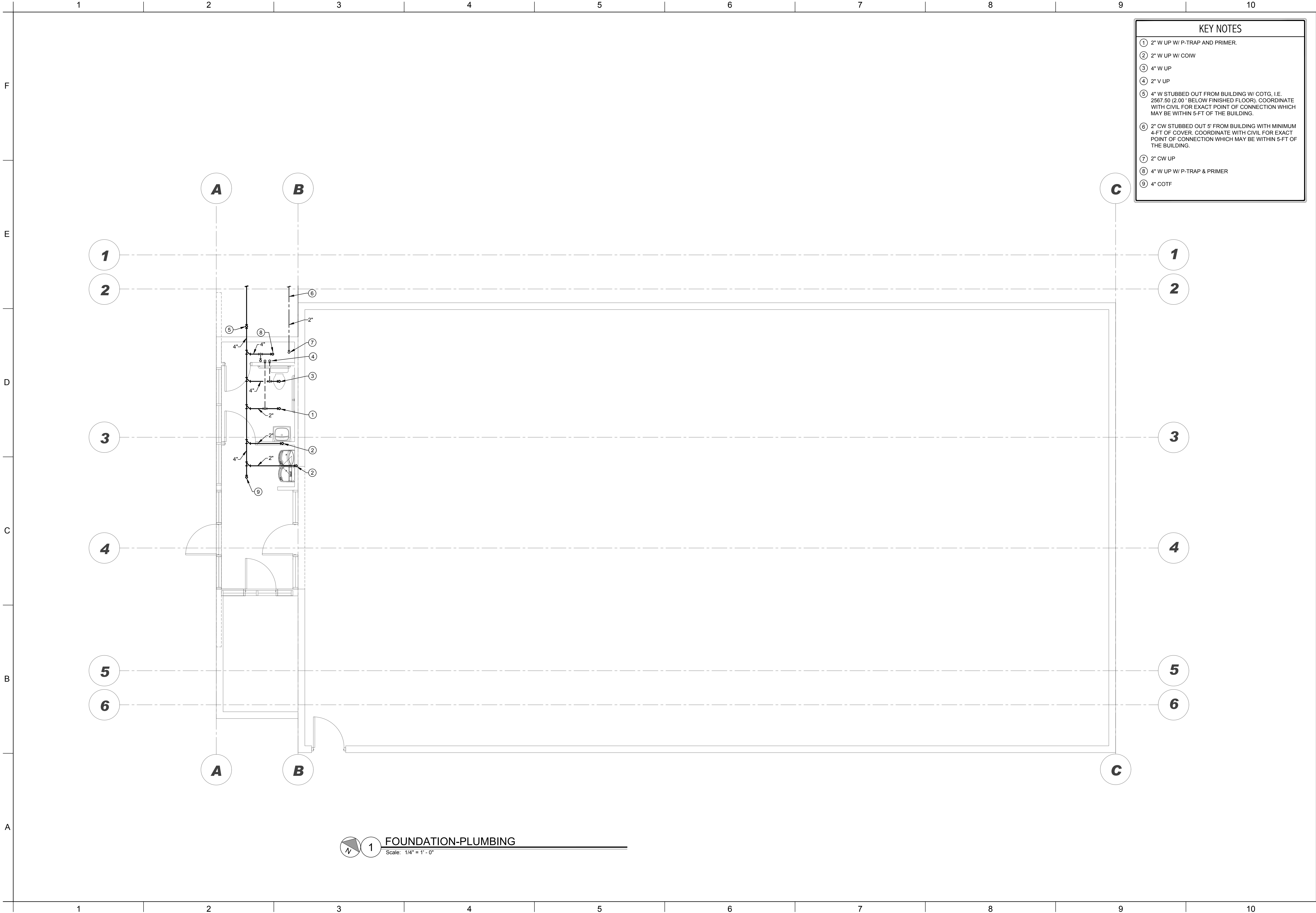
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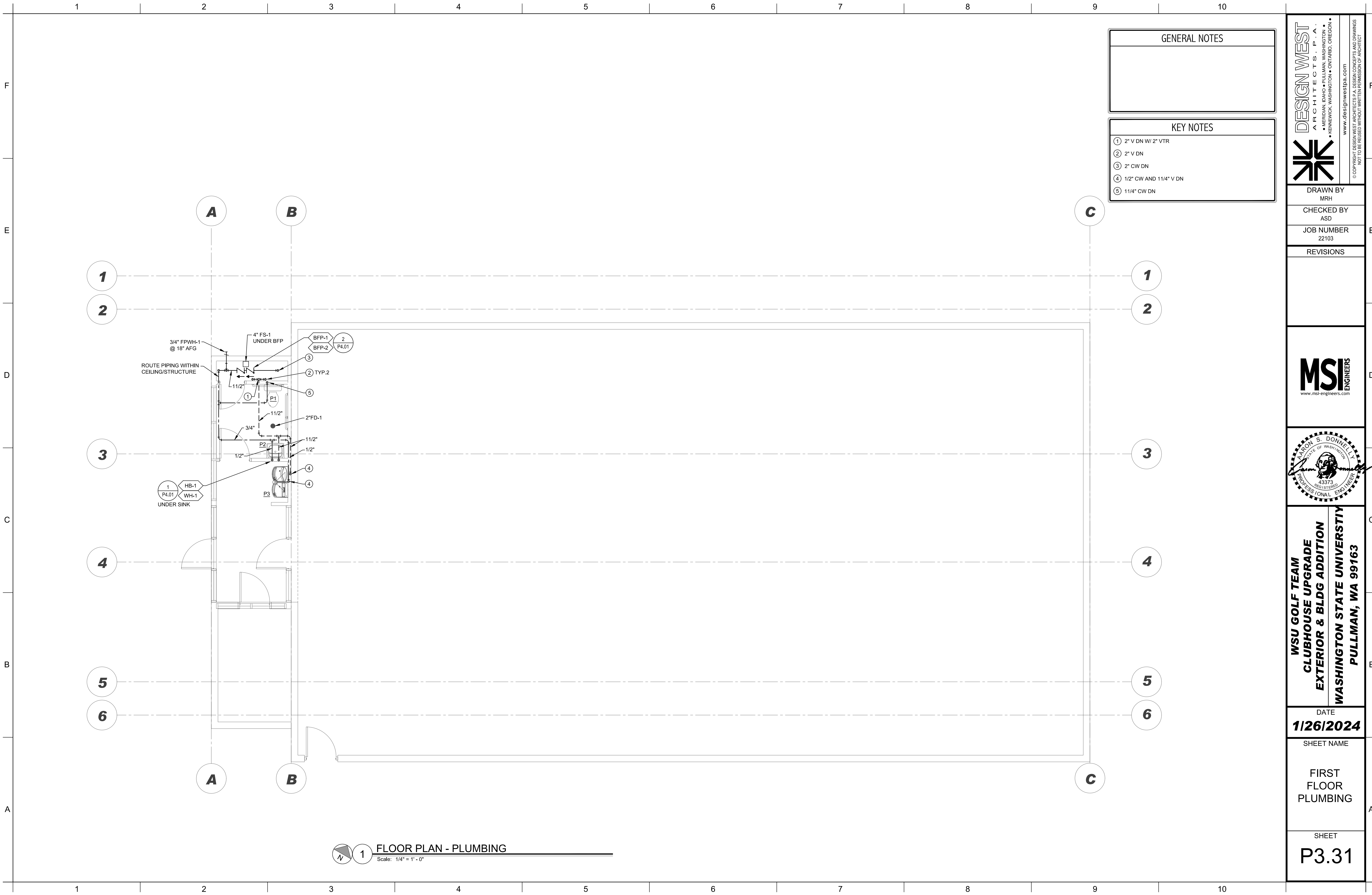
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GENERAL NOTES

KEY NOTES

1

2" V DN W/ 2" VTR

2

2" V DN

3

2" CW DN

4

1/2" CW AND 1 1/4" V DN

5

1 1/4" CW DN

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ARON S. DONNELLY

STATE OF WASHINGTON

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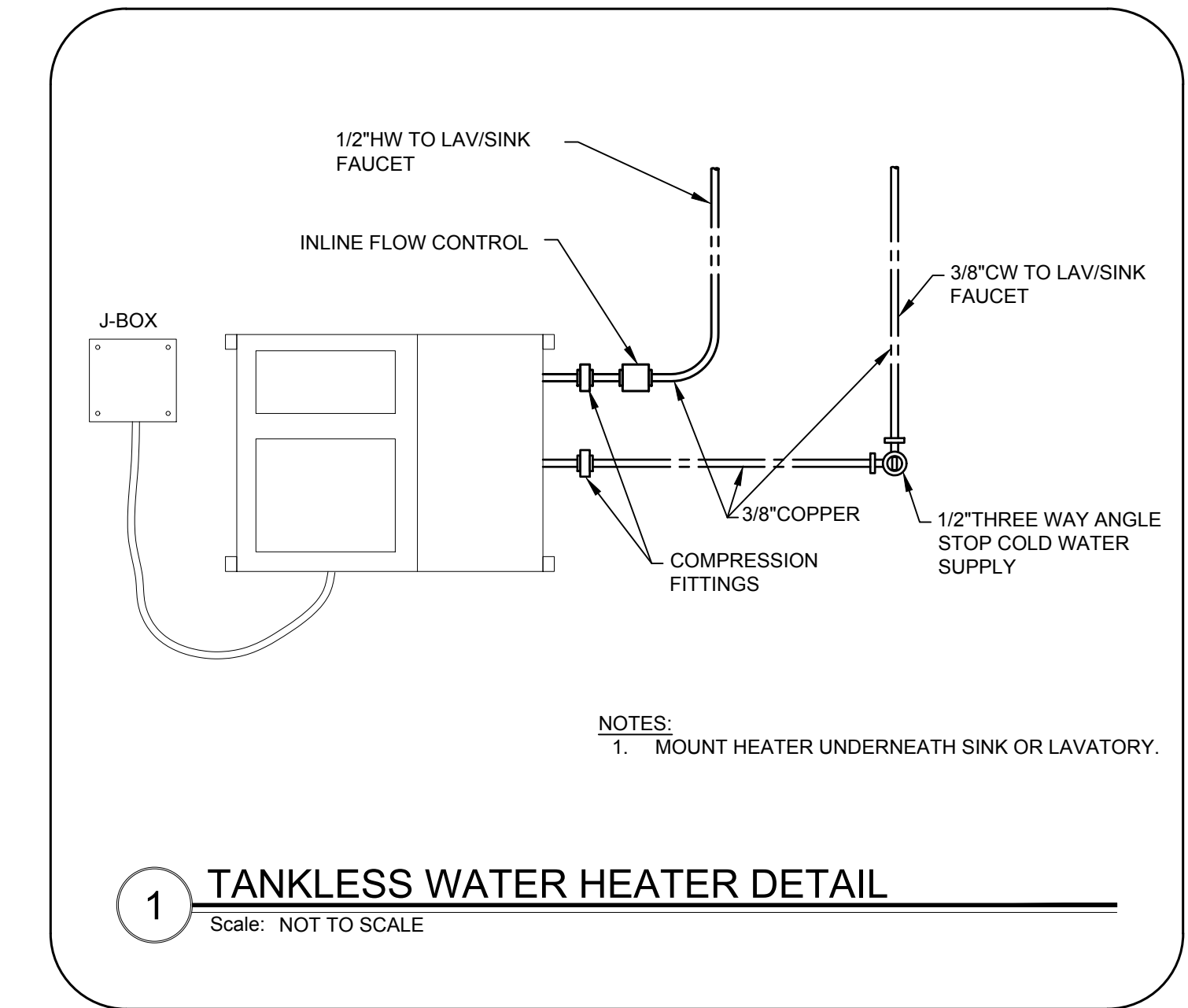
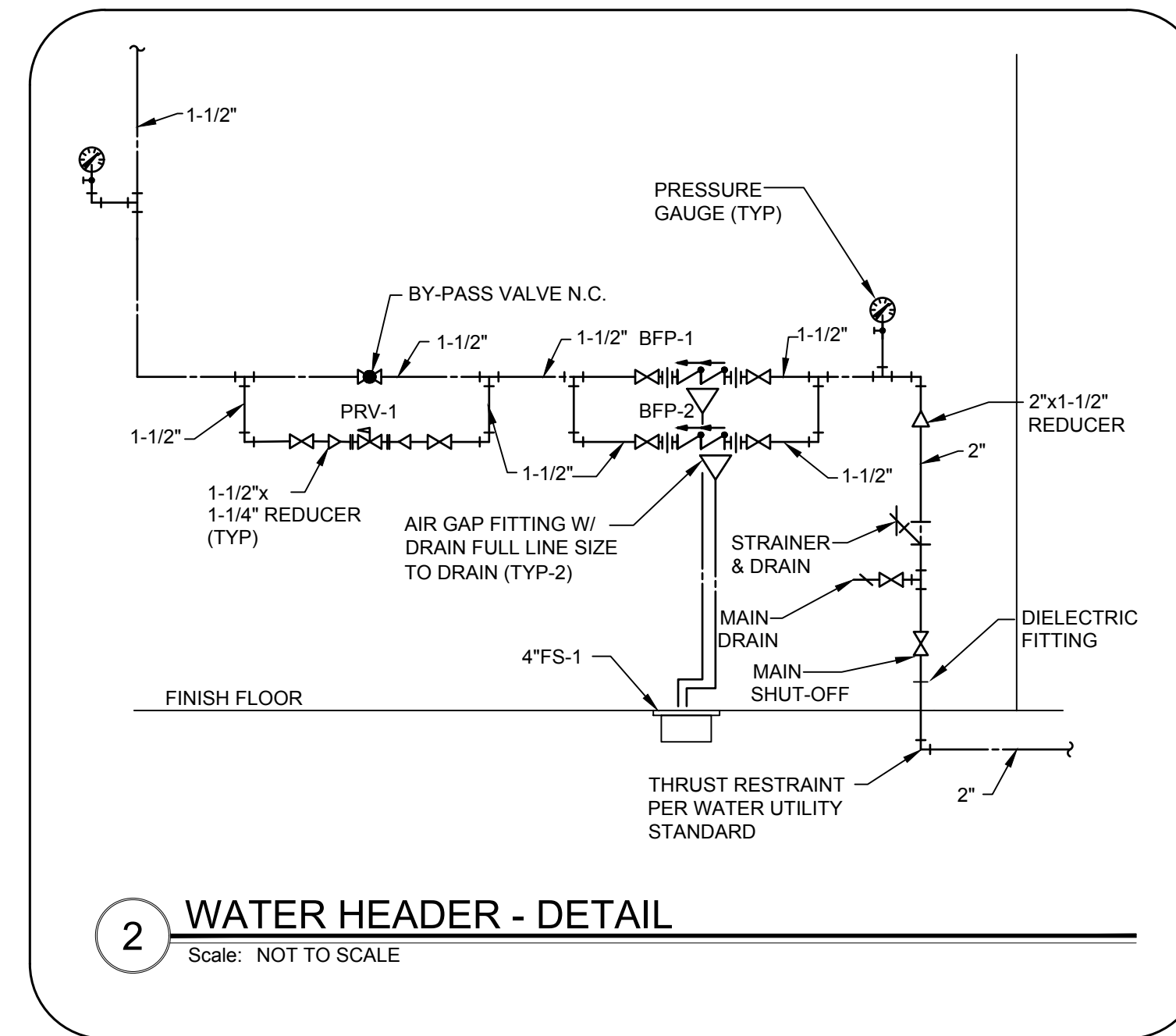
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